

DEVELOPMENT REVIEW APPLICATION

STAFF CONTACT		PROJECT NO(s)			PRE-APPLICATION NO.		
STAFF CONTACT		ELD-24-04			PRE-APPLICATION NO.		
NON-REFUNDABLE FEE(S)	\$4,900	REFUNDABLE DEPOSIT(S)		TOTAL \$4 ,	900		
Type of Review (Please check a	all that apply):						
Annexation (ANX)	Final Pla	at (FP) Related File#	🔲 Sul	bdivision (SUB)			
Appeal (AP)	Flood N	lanagement Area (FMA)	Ter	mporary Uses (M	ISC)		
CDC Amendment (CDC)	=	Review (HDR)	_	☐ Time Extension (EXT)			
Code Interpretation (MISC)		Adjustment (LLA)		ght of Way Vacati	on (VAC)		
Conditional Use (CUP)		artition (MIP)	=	riance (VAR)			
Design Review (DR		ation of Approval (MOD)			Protection/Single Lot (WAP)		
Tree Easement Vacation (MISC)		nforming Lots, Uses & Structures			Protection/Wetland (WAP)		
Expediated Land Division (ELD)		l Unit Development (PUD)			in River Greenway (WRG)		
Extension of Approval (EXT)	☐ Street \	/acation	∐ Zoı	ne Change (ZC)			
Pre-Application, Home Occupation,	Sidewalk Use, A	ddressing, and Sign applications requ	uire differ	rent forms, availa	able on the website.		
Site Location/Address: 4194 Corr	nwall Street, We	est Linn, OR 97068	Assesso	or's Map No.: 21	E36BA		
			Tax Lot	(s): 6500			
			Total La	and Area: 0.46 A	AC (20,037 SF)		
		es to partition the property into three		or middle housi	ng, in accordance with		
the	e City of West L	inn's land development requireme	ents.				
Applicant Name*:TPC PROPER	TY LLC contact	: JJ Portlock		none: (425) 829-			
Address: PO BOX 521			En	nail: JPORTLOCK	(@THEPORTLOCKCO.COM		
City State Zip: WEST LINN, C	R 97068						
Owner Name (required): The Port	llock Company		Ph	none:			
Address: PO Box 521, West	Linn OR 97068		En	nail:			
City State Zip:							
Consultant Name: FIRWOOD DE	SIGN GROUP	LLC contact: Carmen Nemes	Ph	none: (503) 668-	3737		
	RIC COLUMBIA			mail: CN@FIRWC			
City State Zip: TROUTDALE,							

- 1. Application fees are non-refundable (excluding deposit). Applications with deposits will be billed monthly for time and materials above the initial deposit. *The applicant is financially responsible for all permit costs.
- 2. The owner/applicant or their representative should attend all public hearings related to the propose land use.
- 3. A decision may be reversed on appeal. The decision will become effective once the appeal period has expired.
- 4. Submit this form, application narrative, and all supporting documents as a single PDF through the Submit a Land Use Application web page: https://westlinnoregon.gov/planning/submit-land-use-application

The undersigned property owner authorizes the application and grants city staff the **right of entry** onto the property to review the application. Applications with deposits will be billed monthly for time and materials incurred above the initial deposit. The applicant agrees to pay additional billable charges.

Applicant's signature

9/24/24

Date Owner's signature (required)

9/24/24 Date

Partition Application Narrative

Project Name: Cornwall 3 Lot Middle Housing

Site Address: 4194 Cornwall Street, West Linn, OR.

Applicant: TPC PROPERTY LLC

PO BOX 521

West Linn, OR 97068

Contact: JJ Portlock

(425) 829-1566

jportlock@theportlockco.com

Prepared By:



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I. General Project Description

The project site is located at 4194 Cornwall Street, West Linn, OR (Parcel #21E36BA 065000), zoned R10. The current 0.459 acre lot features an existing residential structure and a detached garage. The existing house and garage will be removed, and the driveway will be improved. The site is relatively flat with slopes less than 5%, gently sloping towards the southeastern side. The remainder of the lot is landscaped with mature trees and a retaining landscape wall along the southern property line. A 20-foot access and utility easement is proposed to provide access to all three lots.

The property is currently connected to public utilities—including water, electricity, natural gas, and sewer services—available from Cornwall Street.

II. Application Approval Requests

The client, TPC Property LLC, proposes to partition the parcel (Parcel #1S3E 36C TL 1400) at 4194 Cornwall Street into three parcels for single-family use. A new 14-foot wide, 135-foot long driveway is proposed to serve the first two lots, and it will continue as a 12-foot wide private driveway serving the third lot.

III. Applicable Code Criteria

The subject property is zoned R-10, which permits triplex residential units as an outright use per CDC 11.030.1.b. This proposal includes three detached single units under the provisions of Section 2 of SB 458 for a middle housing land division. The applicable criteria are noted below in **bold** typeface with the applicant's responses in *italic* typeface.

SB 458 Section 2:

- (1) As used in this section, "middle housing land division" means a partition or subdivision of a lot or parcel on which the development of middle housing is allowed under ORS 197.758 (2) or (3).
- (2) A city or county shall approve a tentative plan for a middle housing land division if the application includes:
- (a) A proposal for development of middle housing in compliance with the Oregon residential specialty code and land use regulations applicable to the original lot or parcel allowed under ORS 197.758 (5);

Response: The property is within the R-10 zoning designation and permits a triplex as outright use. The proposal includes a middle housing land division to create three lots, each with a detached single-family unit

(b) Separate utilities for each dwelling unit;

Response: A preliminary street and utility plan shows individual utilities for each lot.

- (c) Proposed easements necessary for each dwelling unit on the plan for:
- (A) Locating, accessing, replacing and servicing all utilities:
- (B) Pedestrian access from each dwelling unit to a private or public road;
- (C) Any common use areas or shared building elements;
- (D) Any dedicated driveways or parking; and
- (E) Any dedicated common area;

Response: A 20-foot shared access and utility easement is proposed, pedestrian access will be allowed via the private access easement. Each parcel has direct access to the shared driveway, with no common areas or shared building elements.

(d) Exactly one dwelling unit per resulting lot, except for common areas.

Response: The proposal includes one unit per lot, as shown in the submitted plans.

(e) Evidence demonstrating how buildings or structures on a resulting lot or parcel will comply with applicable building codes provisions relating to new property lines and, notwithstanding the creation of new lots or parcels, how structures or buildings located on the newly created lots or parcels will comply with the Oregon residential specialty code.

Response: Building setbacks are shown on the site plan. Existing structures will be removed before constructing the triplex units. A building permit will ensure compliance with the Oregon Residential Specialty Code.

- (3) A city or county may add conditions to the approval of a tentative plan for a middle housing land division to:
 - (a) Prohibit the further division of the resulting lots or parcels.
- (b) Require that a notation appear on the final plat indicating that the approval was given under this section.

Response: The applicant understands that conditions may be applied, including noting the approval on the final plat.

- (4) In reviewing an application for a middle housing land division, a city or county:
 - (a) Shall apply the procedures under ORS 197.360 to 197.380.
- (b) May require street frontage improvements where a resulting lot or parcel abuts the street consistent with land use regulations implementing ORS 197.758.
- (c) May not subject an application to approval criteria except as provided in this section, including that a lot or parcel require driveways, vehicle access, parking or minimum or maximum street frontage.
 - (d) May not subject the application to procedures, ordinances or regulations adopted under
- ORS 92.044 or 92.046 that are inconsistent with this section or ORS 197.360 to 197.380.
- (e) May allow the submission of an application for a middle housing land division at the same time as the submission of an application for building permits for the middle housing.
- (f) May require the dedication of right of way if the original parcel did not previously provide a dedication.
 - (5) The type of middle housing developed on the original parcel is not altered by a middle housing
- land division.
- (6) Notwithstanding ORS 197.312 (5), a city or county is not required to allow an accessory dwelling
- unit on a lot or parcel resulting from a middle housing land division.
- (7) The tentative approval of a middle housing land division is void if and only if a final subdivision or partition plat is not approved within three years of the tentative approval. Nothing in this section or ORS 197.360 to 197.380 prohibits a city or county from requiring a final plat before issuing building permits.

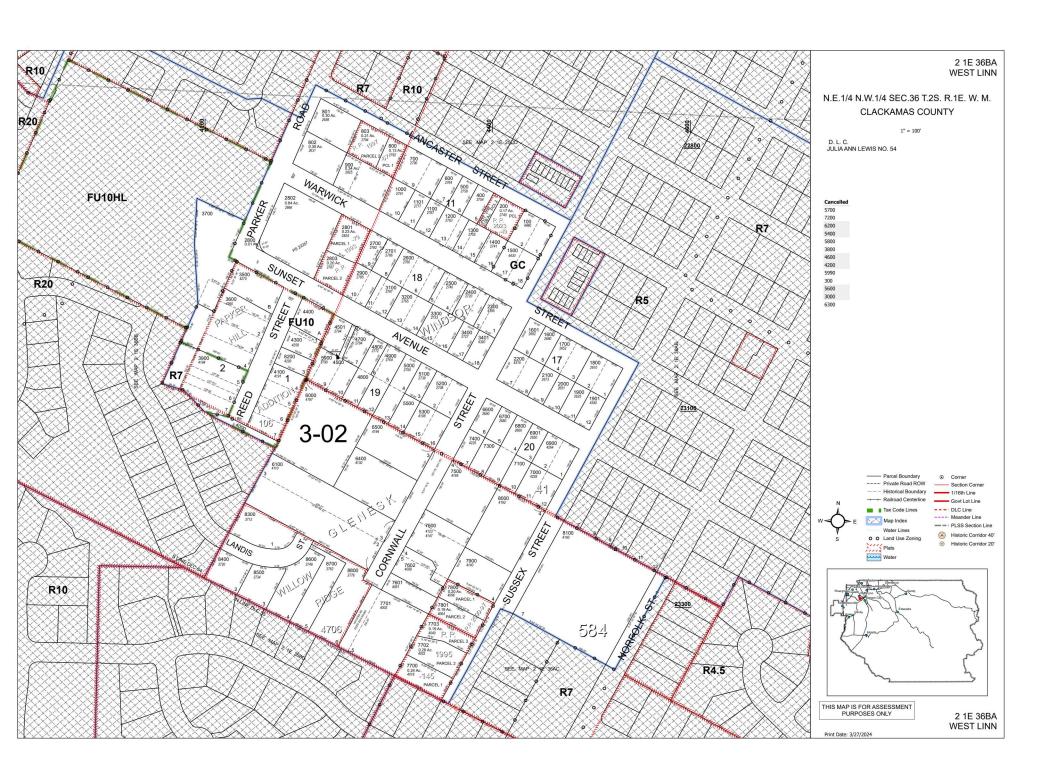
Response: The applicant acknowledges the review process, including possible conditions and requirements and is aware that the application is void if the final plat is not approved within three years.

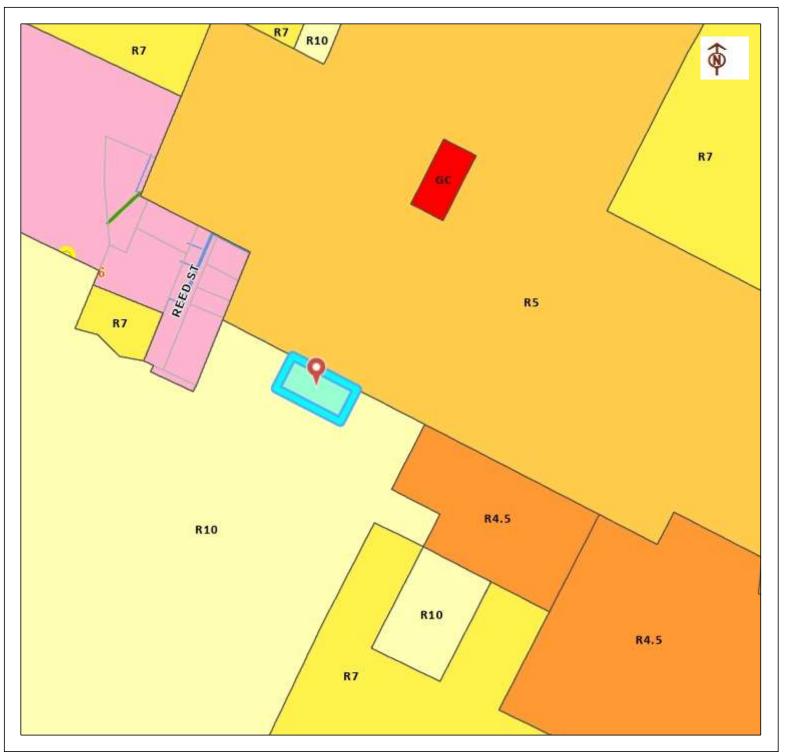
IV. Conclusion

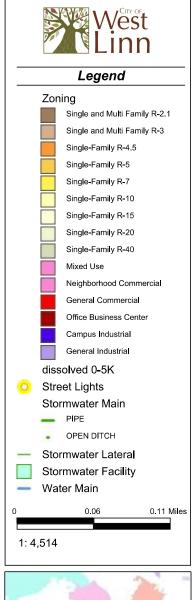
This application narrative, along with the accompanying plans and supporting materials, demonstrates compliance with all applicable provisions of SB 458. The applicant respectfully requests approval of this application by the Planning Department.

V. Items Submitted With The Application

- Clackamas County Assessor Map
- West Linn Zoning Map
- Preliminary Plan Set









Notes

This map was automatically generated using Geocortex Essentials.

CORNWALL - 3 LOT PARTITION MIDDLE HOUSING - TAX LOT 21E36BA06500

DEVELOPMENT ENGINEERING PLANS

LOCATED AT 4194 CORNWALL ST, CITY OF WEST LINN, OREGON 97068

GENERAL NOTES:

- ALL REFERENCES TO THE CITY OF WEST LINN STANDARDS REFER TO THE CURRENT PUBLIC WORKS STANDARD CONSTRUCTION **SPECIFICATIONS**
- 2. THE DESIGN ENGINEER WILL BE RESPONSIBLE FOR INSPECTION OF THE PROPOSED IMPROVEMENTS WITH OVERSIGHT FROM THE CITY OF WEST LINN.
- 3. THE CONTRACTOR SHALL PROVIDE A WORK SCHEDULE TO THE ENGINEER AND CITY AND PROVIDE 24-HOUR NOTICE OF ANY TESTING REQUIRING WITNESSING BY THE CITY OR ENGINEER.
- ANY CHANGES TO THE PLANS MUST RECEIVE APPROVAL BY THE ENGINEER AND CITY IN WRITING BEFORE PROCEEDING WITH THE WORK.
- 5. A PUBLIC IMPROVEMENT GUARANTEE AGREEMENT, A PRE-CONSTRUCTION MEETING, AND INSTALLATION OF THE EROSION CONTROL MEASURES, ARE ALL REQUIRED PRIOR TO BEGINNING CONSTRUCTION. 6. A CITY AND ENGINEER REPRESENTATIVE MUST BE PRESENT FOR ALL TESTING, AND THE CITY SHALL BE FURNISHED A COPY OF ALL TEST
- CONTRACTOR SHALL VERIFY DEPTH AND LOCATION OF EXISTING UTILITIES AND POINTS OF CONNECTION PRIOR TO ORDERING MATERIALS.
- OTHER EXISTING UTILITIES MAY EXIST AND IF DISCREPANCIES ARE FOUND THE CONTRACTOR SHALL NOTIFY THE ENGINEER. THE CONTRACTOR SHALL ERECT AND MAINTAIN TRAFFIC CONTROL PER THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES, PART VI. CONSTRUCTION AND MAINTENANCE, AND SUBMIT A TRAFFIC PLAN TO THE CITY PRIOR TO BEGINNING WORK. FOR TEMPORARY TRAFFIC CONTROL. REFER TO ODOT TEMPORARY TRAFFIC CONTROL MANUAL.
- OREGON LAW REQUIRES THAT THE RULES ADOPTED BY OREGON UTILITY NOTIFICATION CENTER BE FOLLOWED. THOSE RULES ARE SET FORTH IN OAR 952-001-0090. COPIES OF THE RULES ARE AVAILABLE BY CALLING THE CENTER OR ACCESSING VIA INTERNET AT WWW.CALLBEFOREYOUDIG.ORG, PORTLAND METRO AREA 503-248-6699.

STREET NOTES:

- ALL STREET SECTIONS TO BE GRUBBED AND GRADED TO A MINIMUM OF 8-INCHES BELOW THE SUBGRADE.
- THE STREET SUBGRADE SHALL CONFORM TO DIVISION 501 OF THE CITY OF WEST LINN STANDARD CONSTRUCTION SPECIFICATIONS. AREAS TO RECEIVE FILL ARE TO BE INSPECTED BY THE CITY OF WEST LINN PERSONNEL AND PROJECT GEOTECHNICAL PRIOR TO PLACEMENT OF FILL. THE PROJECT GEOTECHNICAL SHALL TEST FOR COMPACTION PER DIVISION 501.03.08 OF THE WEST LINN STANDARD CONSTRUCTION
- AGGREGATE BASE ROCK SHALL CONFORM TO THE REQUIREMENTS OF DIVISION 205 OF THE WEST LINN STANDARD CONSTRUCTION SPECIFICATIONS. BASE COURSE IS 1 1/2" - 0" CRUSHED ROCK AND LEVELING COURSE IS 3/4" - 0" CRUSHED ROCK. THE CITY OF WEST LINN REQUIRES A PROOF ROLL WITH A LOADED 10-CUBIC YARD OR LARGEST USED (IE SUPER SOLO) DUMP TRUCK OF THE SUBGRADE PRIOR TO PLACEMENT OF THE BASE ROCK AND AT TOP OF ROCK JUST PRIOR TO PAVING.
- ASPHALT CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF WEST LINN STANDARD CONSTRUCTION SPECIFICATIONS, DIVISION 205 THE BASE LIFT SHALL BE CLASS "B" AC AND THE TOP LIFT SHALL BE CLASS "C" AC MEETING THE WEST LINN STANDARD CONSTRUCTION SPECIFICATIONS, DIVISION 505. THE TOP LIFT OF ASPHALT CONCRETE SHALL NOT BE PLACED PRIOR TO RECEIVING AUTHORIZATION FROM THE CITY OF WEST LINN.
- CURB AND GUTTER SHALL HAVE A COMPRESSIVE STRENGTH OF 3300 PSI, AND MAXIMUM AGGREGATE PER DIVISION 205 OF THE WEST LINN STANDARD CONSTRUCTION SPECIFICATIONS. A PROOF ROLL OF THE CURB LINES IS REQUIRED PRIOR TO CURB PLACEMENT. CONTRACTION JOINTS ARE TO BE INSTALLED AT 15-FOOT MAXIMUM. THE CONTRACTOR IS TO STAMP LOCATION OF SEWER AND WATER LINES CROSSING THE CURB LINE WITH AN "S" OR "W".

STORM SEWER NOTES:

- RIBBED PVC ASTM 794 OR DUCTILE IRON CONFORMING TO ASTM C 151, CLASS 52, WITH RUBBER JOINTS REQUIRED. GUTTER INLETS SHALL BE POURED IN PLACE CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH OF 3300 PSI. FRAMES SHALL BE FABRICATED OF STRUCTURAL STEEL, ASTM A-3733.
- MANHOLE BASES MAY BE POURED IN PLACE CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH OF 3300 PSI OR PRECAST. MANHOLE RISERS AND TOPS SHALL BE PRECAST WITH MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI. TOPS SHALL BE ECCENTRIC CONES EXCEPT WHERE THERE IS INSUFFICIENT HEADROOM REQUIRES A FLAT TOP. THE INTERIOR DIMENSIONS NOTED ON THE PLANS ARE MINIMUMS. AND SOME OR ALL OF THE MANHOLES MAY BE REQUIRED TO BE OVERSIZED. THE CONTRACTOR SHALL VERIFY WITH THE MANHOLE MANUFACTURER FOR THE ACTUAL SIZE OF MANHOLE NEEDED FOR TYPE AND SIZE OF PIPE TO BE USED. INVERTS SHALL BE CONSTRUCTED SO AS TO PROVIDE A SMOOTH FLOW AND PIPES SHALL BE CONNECTED TO THE MANHOLE BY MEANS OF A FLEXIBLE CONNECTION AND SHALL HAVE A SHEAR JOINT LOCATED 18-INCHES OUTSIDE THE MANHOLE.
- MANHOLES LOCATED IN EASEMENTS REQUIRE A TAMPER PROOF FRAME AND COVER AND SET 12-INCHES ABOVE FINISHED GRADE IN UNPAVED AREAS.
- GRANULAR BACKFILL (3/4" 0") IS TO BE COMPACTED TO 95% OF MAXIMUM DRY DENSITY PER AASHTO T-180 TEST METHOD AND NATIVE MATERIAL SHALL BE COMPACTED TO 95% OF IN-PLACE DRY DENSITY OF SURROUNDING SOIL.
- 6. A VIDEO INSPECTION IN ACCORDANCE WITH DIVISION 601.03.11 PER THE WEST LINN STANDARD CONSTRUCTION SPECIFICATIONS IS
- REQUIRED. ALL TESTS SHALL BE WITNESSED BY THE ENGINEER AND A REPRESENTATIVE OF THE CITY. 7. A PLUMBING PERMIT IS REQUIRED FOR ALL STORM DRAINS BEYOND THE FIRST CLEANOUT.

SANITARY SEWER NOTES:

- PIPE SHALL BE PVC SEWER PIPE CONFORMING TO ASTM D-3034 SDR 35, MINIMUM STIFFNESS SHALL BE 46 PSI AND JOINT TYPE SHALL BE ELASTOMERIC GASKET CONFORMING TO ASTM D-3212.
- MANHOLE BASE MAY BE POURED IN PLACE CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH OF 3300 PSI OR PRECAST. MANHOLE RISERS AND TOPS SHALL BE PRECAST WITH MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI. TOPS SHALL BE ECCENTRIC CONES EXCEPT WHERE THERE IS INSUFFICIENT HEADROOM REQUIRES A FLAT TOP. INVERTS SHALL BE CONSTRUCTED TO PROVIDE A SMOOTH FLOW WITH THE CHANNEL BEING ABLE TO PAS A 6" X 36" CYLINDER INTO THE PIPES. PVC PIPE SHALL BE CONNECTED TO THE MANHOLE BY MEANS OF A FLEXIBLE CONNECTION AND SHALL HAVE A SHEAR JOINT LOCATED 18-INCHES OUTSIDE THE MANHOLE. CEMENT GROUT FOR CONNECTING PVC SEWER PIPE WILL NOT BE PERMITTED.
- 3. ALL MANHOLES LOCATED IN EASEMENT AREAS WILL HAVE TAMPER PROOF FRAMES AND COVERS WITH THE COVER SET 12-INCHES ABOVE FINISH GRADE IN UNPAVED AREAS.
- 4. GRANULAR BACKFILL (3/4" 0") IS TO BE COMPACTED TO 95% OF MAXIMUM DRY DENSITY PER AASHTO T-180 TEST METHOD AND NATIVE MATERIAL SHALL BE COMPACTED TO 95% OF IN-PLACE DRY DENSITY OF SURROUNDING SOIL. EXCAVATION, BEDDING, AND BACKFILL SHALL BE IN CONFORMANCE WITH DIVISION 204 OF THE WEST LINN STANDARD CONSTRUCTION SPECIFICATIONS. BACKFILL WITHIN NEW AND EXISTING STREETS SHALL BE CLASS B.
- 5. PVC SERVICE LATERALS SHALL BE 4-INCH CONFORMING TO THE SAME MATERIALS AS THE MAINLINE SEWERS. SERVICE LATERALS SHALL BE INSTALLED TO A POINT BEYOND THE UTILITY EASEMENT AS SHOWN ON THE PLAN. THE SERVICE LATERALS SHALL BE PLUGGED WITH THE LOCATION MARKED WITH A 2X4 PAINTED GREEN.
- SANITARY SEWER PIPE, INCLUDING SERVICE LATERALS, SHALL BE TESTED IN ACCORDANCE WITH WEST LINN STANDARD CONSTRUCTION SPECIFICATIONS DIVISION 301.03.09 AND MANHOLES SHALL BE VACUUM TESTED IN ACCORDANCE WITH DIVISION 302.03.07. TESTS SHALL BE WITNESSED BY THE ENGINEER AND THE CITY OF WEST LINN. CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE TESTING AND PROVIDING PASSING TESTS PRIOR TO CONNECTION TO THE EXISTING SYSTEM.
- 7. A PLUMBING PERMIT FROM THE CITY IS REQUIRED FOR SANITARY SEWER LATERALS BEYOND THE FIRST CLEANOUT.
- 8. ALL MATERIALS, INSTALLATION, TESTING, AND INSPECTIONS TO BE IN STRICT ACCORDANCE WITH THE CITY OF WEST LINN PUBLIC WORKS STANDARDS AND THE DEQ.

PROJECT LOCATION Plano, Voice, and Guitar Lesso West Linn Public Works Department

PROJECT CIVIL ENGINEER

KELLI GROVER, PE FIRWOOD DESIGN GROUP, LLC 359 E. HISTORIC COLUMBIA RIVER HWY TROUTDALE, OR 97060 PHONE (503) 668-3737

PROJECT CONTRACTOR

PHONE (425) 829-1566

THE PORTLOCK COMPANY LLC 4194 CORNWALL ST WEST LINN, OR PROJECT CONTACT - JJ PORTLOCK JPORTLOCK@THEPORTLOCKCO.COM

PROJECT SURVEYOR

CMT SURVEYING AND CONSULTING 20330 SE HIGHWAY 212 DAMASCUS, OR 97089 PHONE (503) 850-4672

PROJECT OWNER

CHRISTMAN, EARL & JENNIFER 14495 S BLUE VISTA DR. OREGON CITY, 97045

PROPOSED LEGEND

PROPOSED MAJOR CONTOUR PROPOSED MINOR CONTOUR PROPOSED SIGHT LINE PROPOSED ROAD CENTER LINE PROPOSED SAWCUT LINE PROPOSED GRAVEL PROPOSED FILTER STRIP PROPOSED EARTHEN SHOULDER PROPOSED AC

PROPOSED CULVERT

PROPOSED UNDERGROUND POWER — UGE — UGE — UGE —

SHEET INDEX						
SHEET NUMBER	SHEET TITLE					
1	COVER SHEET					
2	EXISTING CONDITIONS & DEMO PLAN					
3	SITE PLAN					
4	GRADING STORM & ESC PLAN					
5	PRELIMINARY UTILITY PLAN					
6	PRELIMINARY PLAT					
7	DETAILS					
8	DETAILS					

LEGAL DESCRIPTION:

TAX LOT 21E36BA06500

N.E. ¼ N.W. ¼ SEC. 36 T.2S. R.1E. W.M. CITY OF WEST LINN, CLACKAMAS COUNTY, OREGON

WATER NOTES:

- 1. WATER MAINS SHALL BE DUCTILE IRON PIPE CONFORMING TO AWWA C151, CLASS 52. PIPE IS TO HAVE CEMENT MORTAR LINING AND BITUMINOUS SEAL COAT CONFORMING TO AWWA C104. JOINTS ARE TO BE PUSH-ON RUBBER GASKETED JOINTS UNLESS OTHERWISE NOTED ON THE PLANS. PIPE FITTINGS ARE TO BE THE SAME MATERIAL AND CLASS AS THE PIPE AND OF DOMESTIC ORIGIN.
- 2. WATER MAINS TO HAVE A MINIMUM COVER OF 36-INCHES.
- 3. ALL TEES, BENDS (HORIZONTAL AND VERTICAL), OR SIGNIFICANT CHANGES IN DIRECTION IN ALIGNMENT SHALL BE RESTRAINED WITH FIELD LOCK GASKETS AND MEGA LUG FITTINGS (SEE SPEC 4.0014).
- 4. GRANULAR BACKFILL (3/4" 0") IS TO BE COMPACTED TO 95% OF MAXIMUM DRY DENSITY PER AASHTO T-180 TEST METHOD AND NATIVE MATERIAL SHALL BE COMPACTED TO 95% OF IN-PLACE DRY DENSITY OF SURROUNDING SOIL. EXCAVATION, BEDDING, AND BACKFILL SHALL BE IN CONFORMANCE WITH DIVISION 204 OF THE WEST LINN STANDARD CONSTRUCTION SPECIFICATIONS. BACKFILL WITHIN NEW AND EXISTING STREETS SHALL BE CLASS B.
- 5. SERVICES SHALL BE 1-INCH TYPE K COPPER. CORPORATION STOPS SHALL BE MUELLER B-25008 OR APPROVED EQUAL. ANGLE STOPS SHALL BE MUELLER B-24258 OR APPROVED EQUAL. METER BOXES SHALL BE DFW PLASTICS MODEL DFW846WBC OR APPROVED EQUAL. METER BOXES ARE TO BE INSTALLED 2 1/2" FROM THE CURB IN PLANTER STRIPS OR BEHIND SIDEWALK. METER BOX LOCATION TO BE
- 6. ALL WATER LINES SHALL BE PRESSURE TESTED AND BIOLOGICALLY TESTED BEFORE CONNECTION TO THE CITY WATER SYSTEM CHLORINATION SHALL BE PER DIVISION 403.13 OF THE CITY OF WEST LINN CONSTRUCTION SPECIFICATIONS. PRESSURE TESTING SHALL BE IN ACCORDANCE WITH THE CITY OF WEST LINN STANDARD CONSTRUCTION SPECIFICATIONS, DIVISION 403.14. TEST PRESSURE WILL BE 180 PSI OR 1.5 TIMES THE NORMAL WORKING PRESSURE, WHICHEVER IS HIGHER.
- 7. CONNECTION OF THE NEW PIPE SYSTEMS WILL NOT BE MADE UNTIL ALL TESTS ARE MADE AND ACCEPTED BY THE CITY OF WEST LINN. 8. A PLUMBING PERMIT IS REQUIRED FOR EXTENSIONS OF WATERLINES BEYOND THE METER.
- 9. ALL MATERIALS, INSTALLATION, AND TESTS TO BE IN STRICT ACCORDANCE WITH THE CITY OF WEST LINN PUBLIC WORKS STANDARD CONSTRUCTION SPECIFICATIONS DIVISION 204, AND THE OREGON STATE HEALTH DIVISION ADMINISTRATION RULES, CHAPTER 333.
- 10. ALL SURPLUS APPURTENANCES SHALL BE RETURNED TO THE CITY OF WEST LINN WATER DEPARTMENT 11. SERVICES TO BE INSTALLED BY WEST LINN STAFF. METERS TO BE PLACED IN PLANTER STRIP OR BACK OF SIDEWALK (NOT IN CONCRETE).

PROJECT DATUM

THE BASIS OF BEARINGS WAS PER SURVEY NO. 2014-039, CLACKAMAS COUNTY RECORDS

NOTICE TO EXCAVATORS:

ATTENTION: OREGON LAW REQUIRES YOU TO FOLLOW RULES ADOPTED BY THE OREGON UTILITY NOTIFICATION CENTER. THOSE RULES ARE SET FORTH IN OAR 952-001-0001 THROUGH OAR 952-001-0090. YOU MAY OBTAIN COPIES OF THE RULES BY CALLING THE CENTER. (NOTE: THE TELEPHONE NUMBER FOR THE OREGON UTILITY

NOTIFICATION CENTER IS (503)-232-1987). POTENTIAL UNDERGROUND FACILITY OWNERS

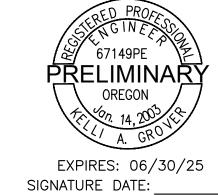
Dig | Safely.

CALL THE OREGON ONE-CALL CENTER 1-800-332-2344 OR 811

EMERGENCY TELEPHONE NUMBERS

NW NATURAL GAS CLACKAMAS COUNTY COMCAST CABLE VERIZON

503-464-7777 503-220-2415 503-742-4400 503-617-1212 800-837-4966



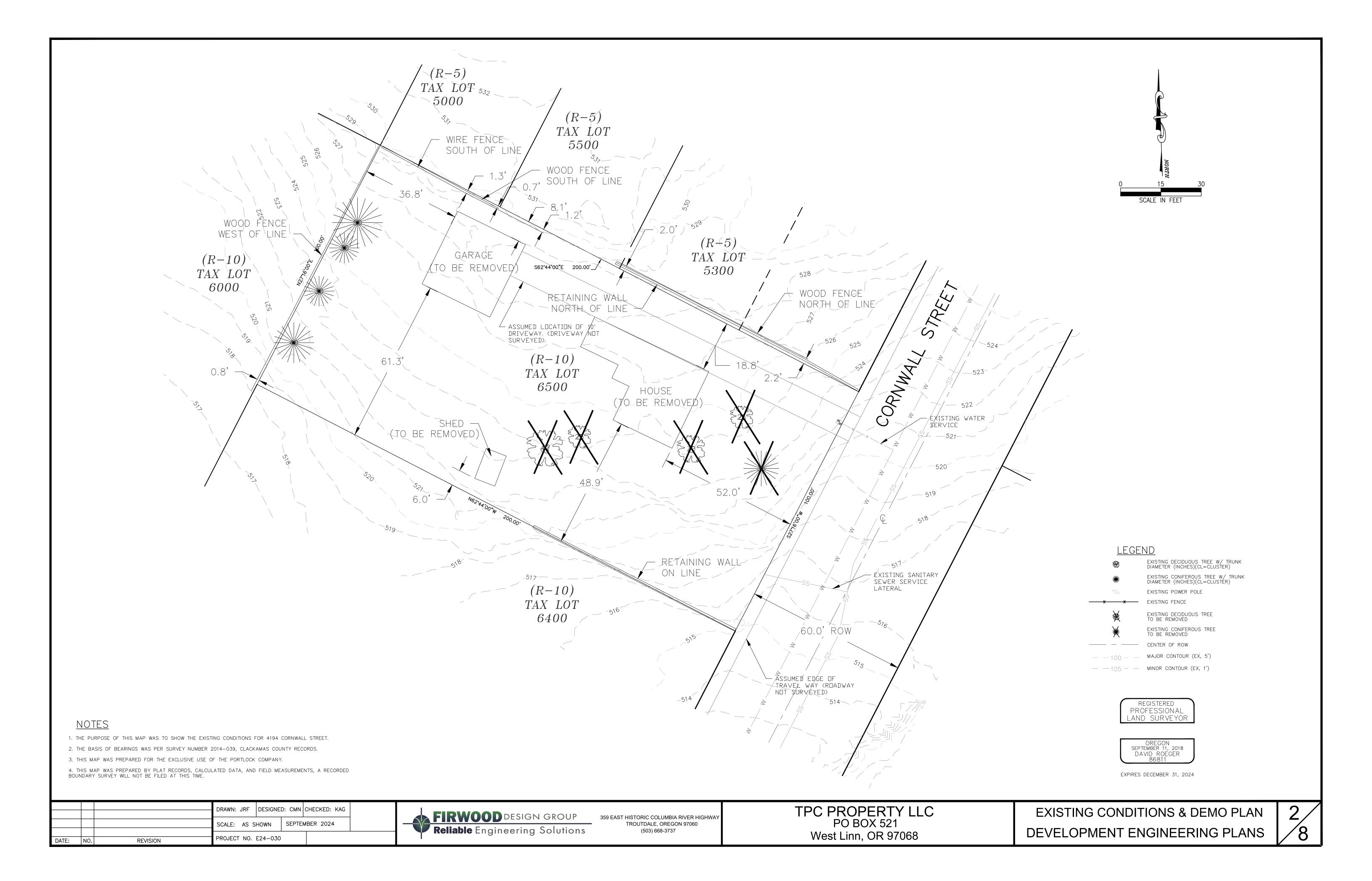
DRAWN: JRF | DESIGNED: CMN | CHECKED: KAG SCALE: AS SHOWN SEPTEMBER 2024 PROJECT NO. E24-030 NO. REVISION



359 EAST HISTORIC COLUMBIA RIVER HIGHWAY TROUTDALE, OREGON 97060 (503) 668-3737

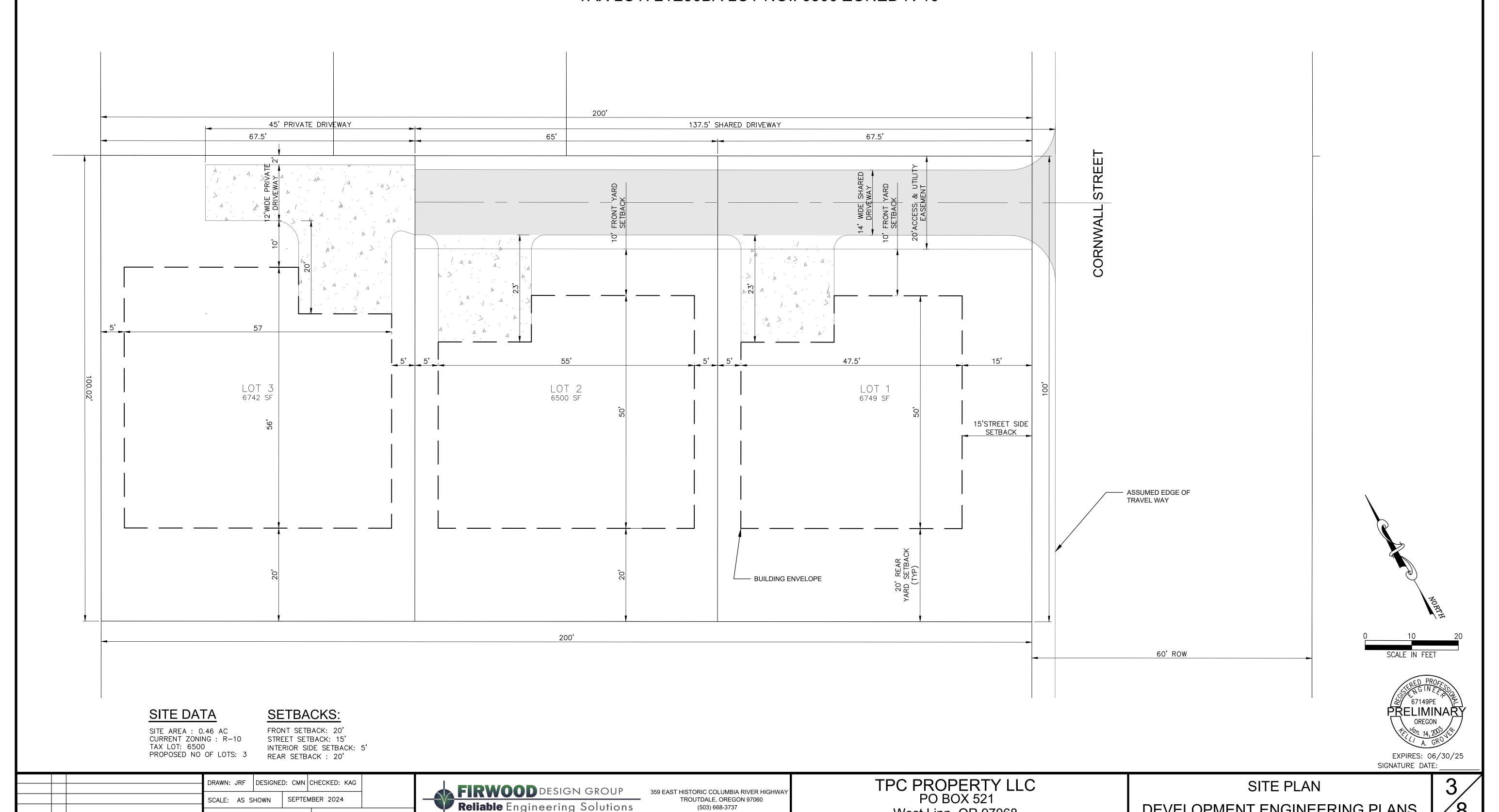
TPC PROPERTY LLC PO BOX 521 West Linn, OR 97068

COVER SHEET DEVELOPMENT ENGINEERING PLANS



CORNWALL - 3 LOT PARTITION MIDDLE HOUSING DEVELOPMENT ENGINEERING PLANS

LOCATED AT 4194 CORNWALL STREET, WEST LINN, OREGON 97068 TAX LOT: 21E36BA LOT NO.: 6500 ZONED R-10



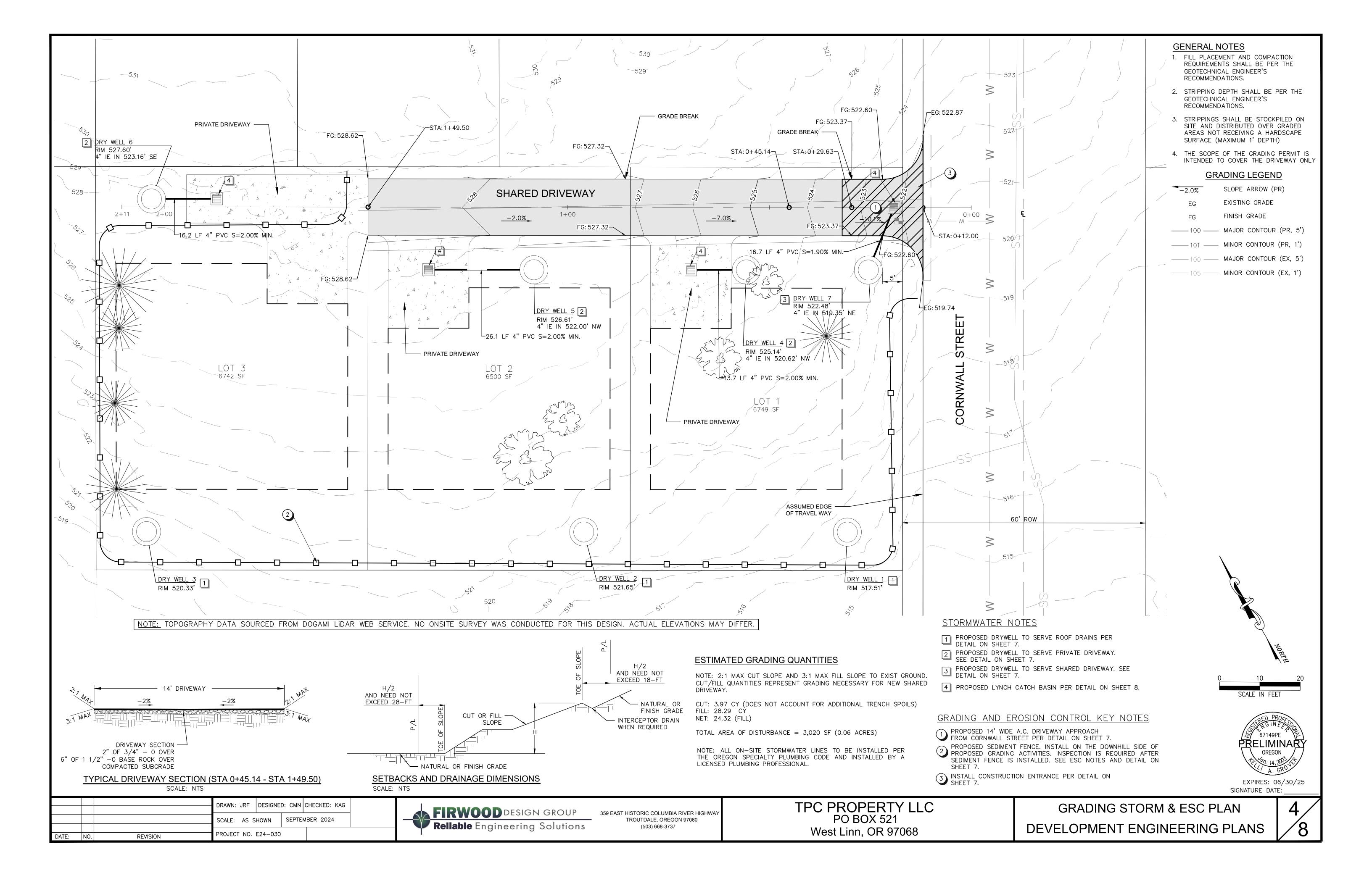
West Linn, OR 97068

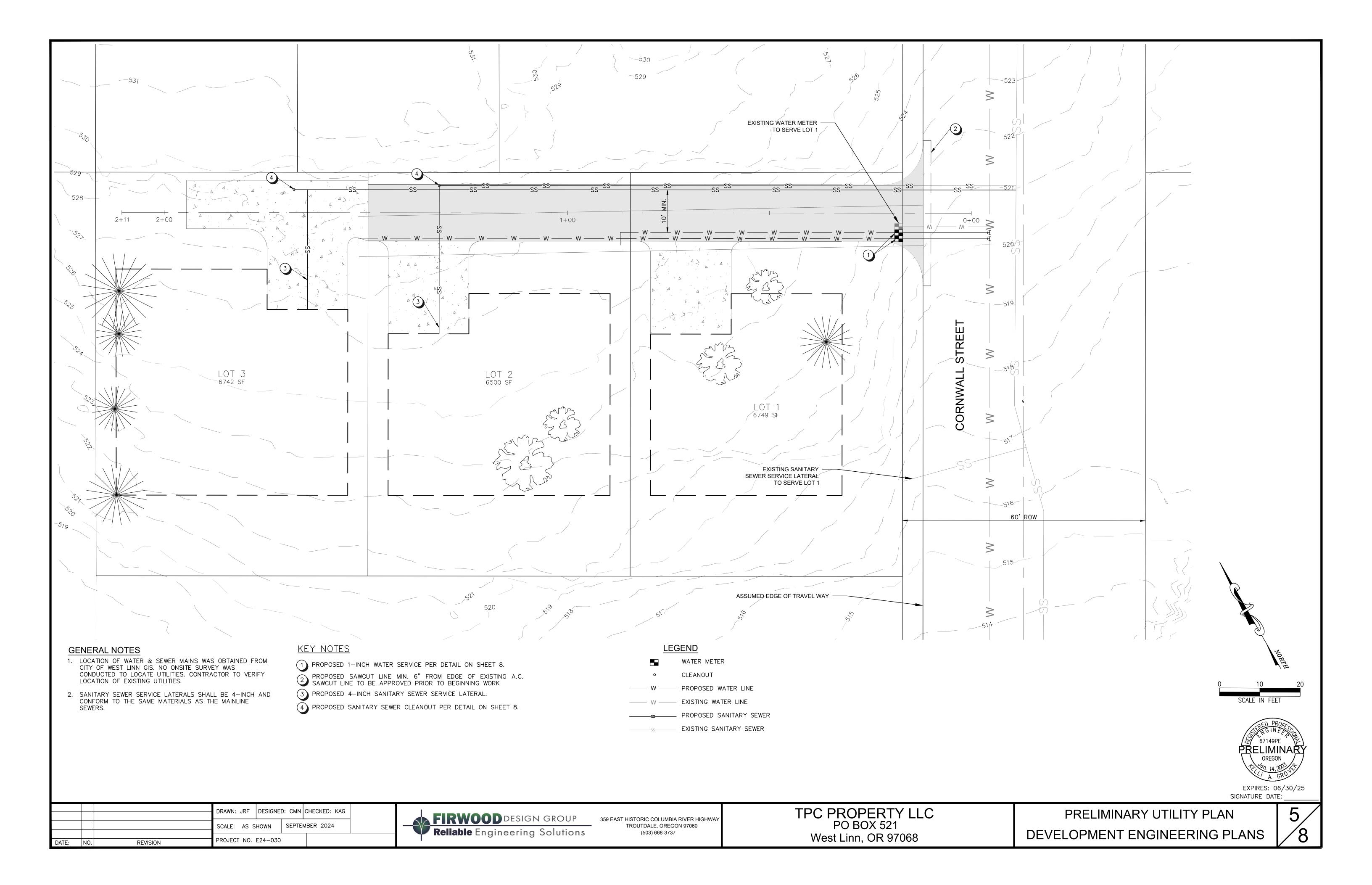
DEVELOPMENT ENGINEERING PLANS

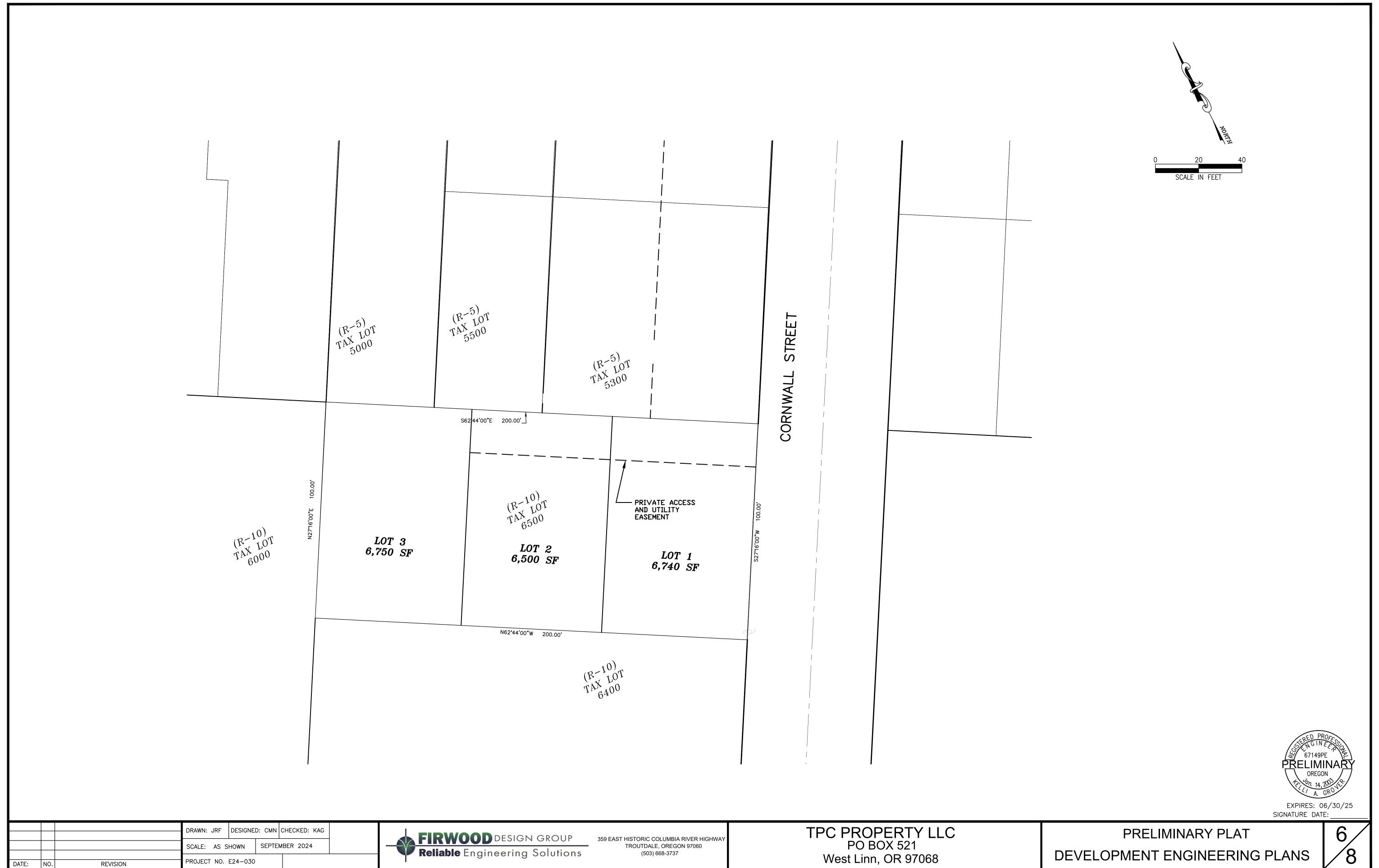
Reliable Engineering Solutions

PROJECT NO. E24-030

REVISION

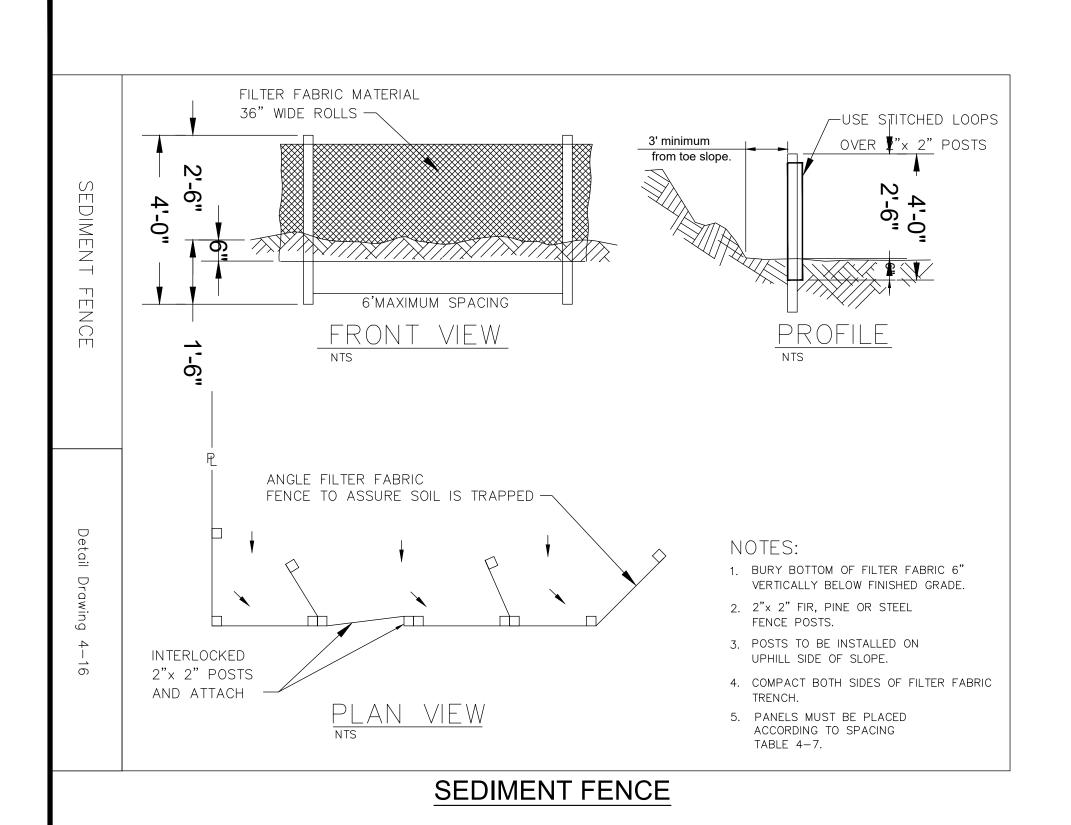


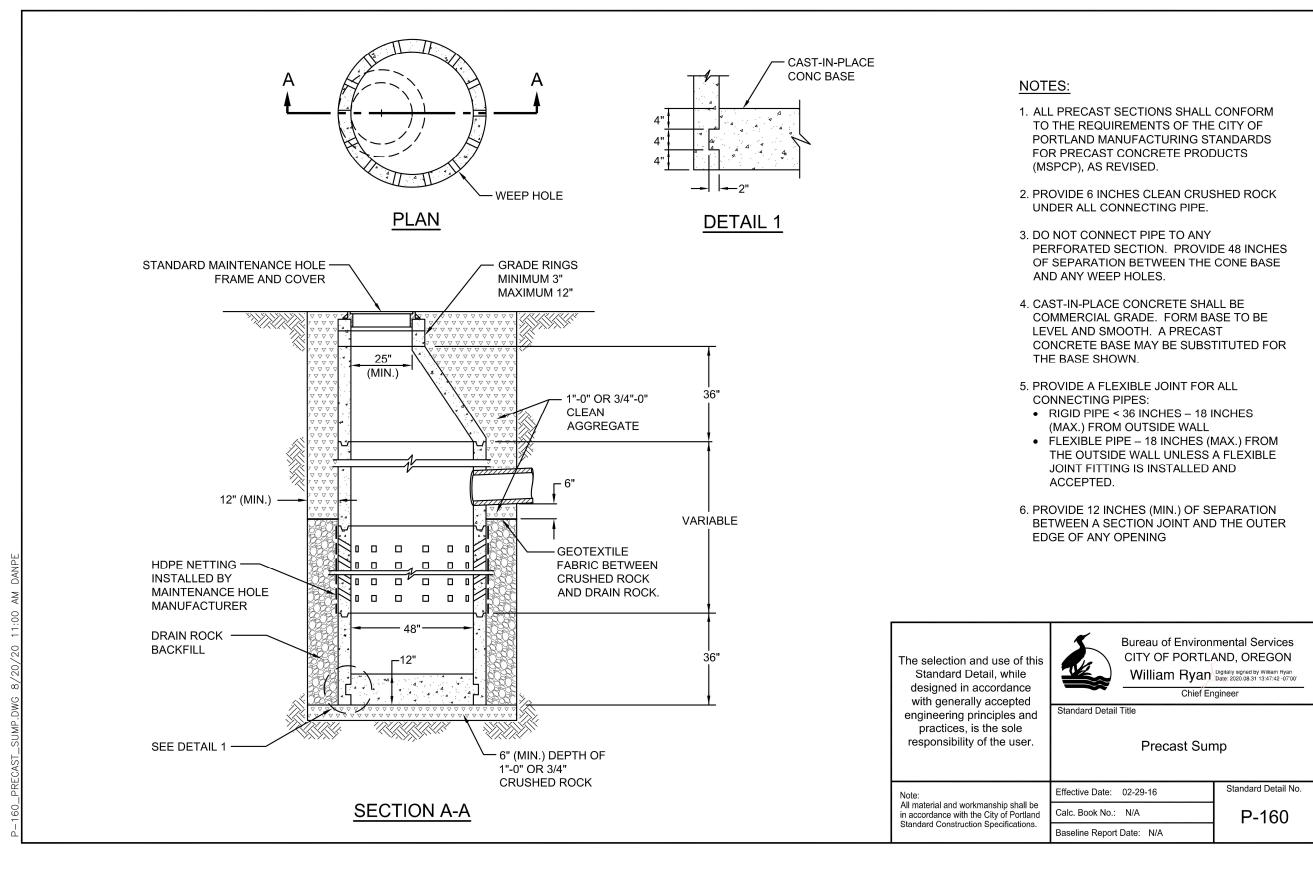


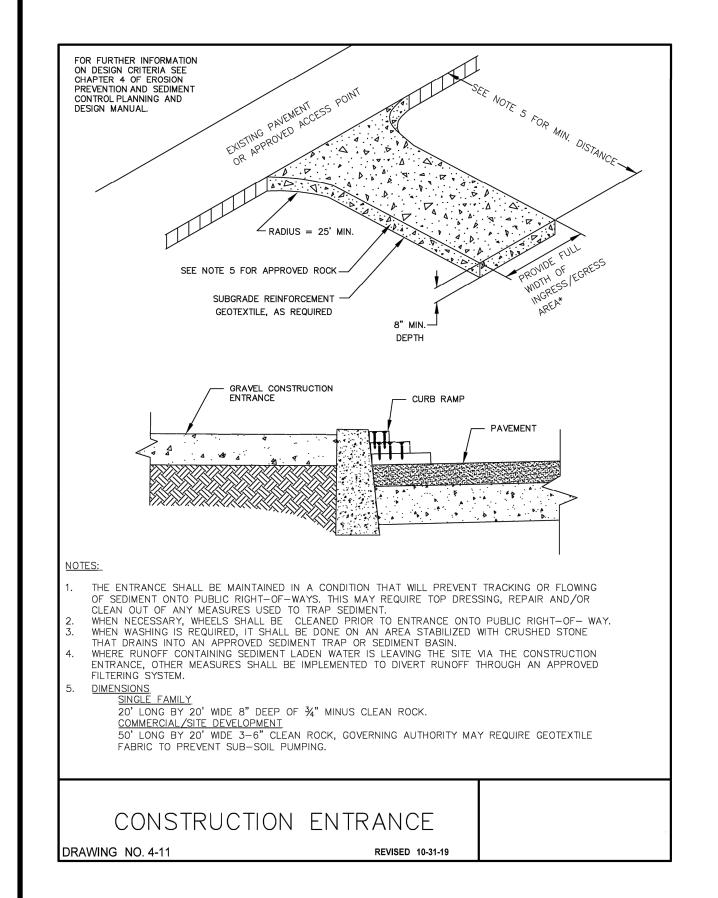


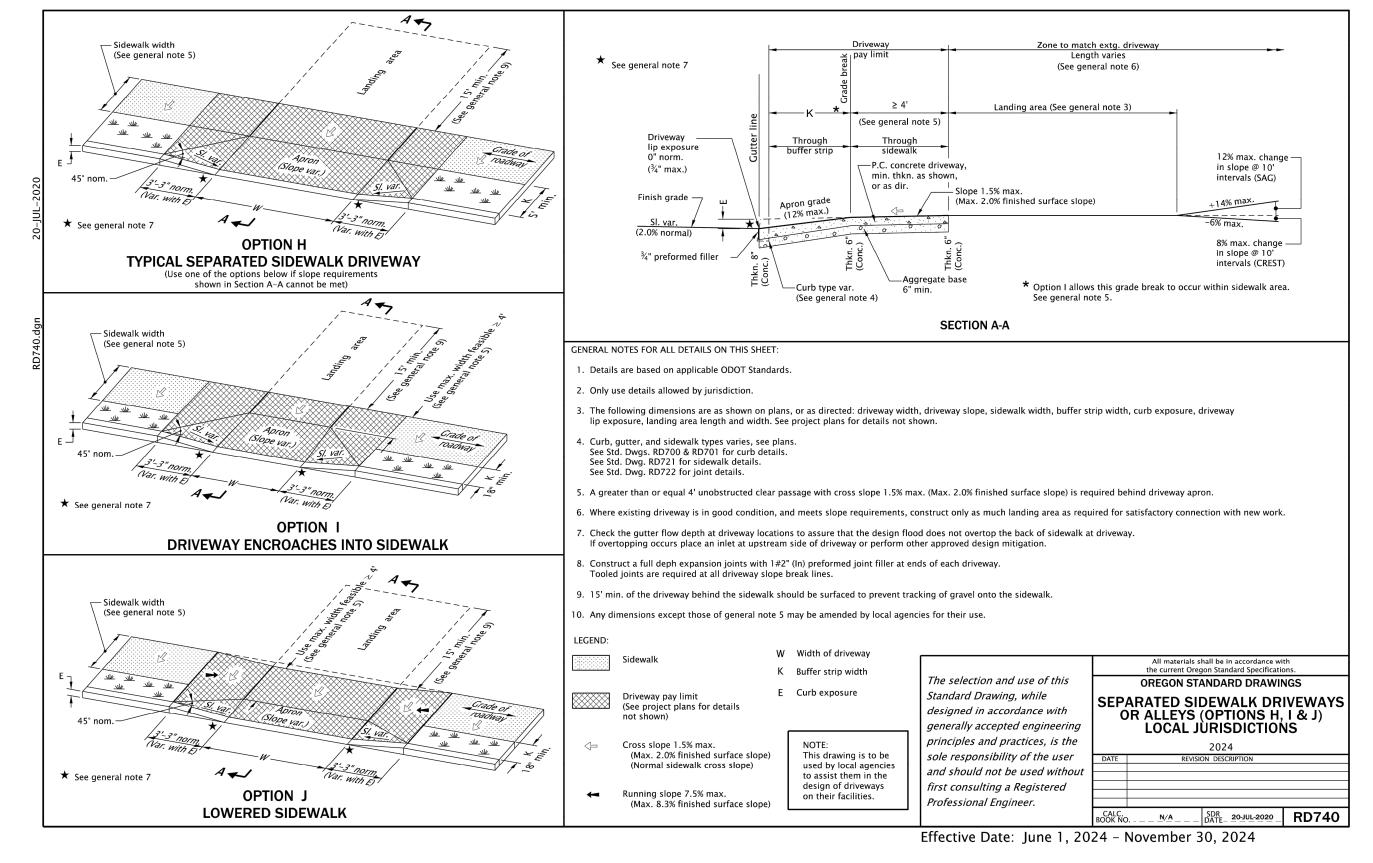
DATE: NO.

REVISION









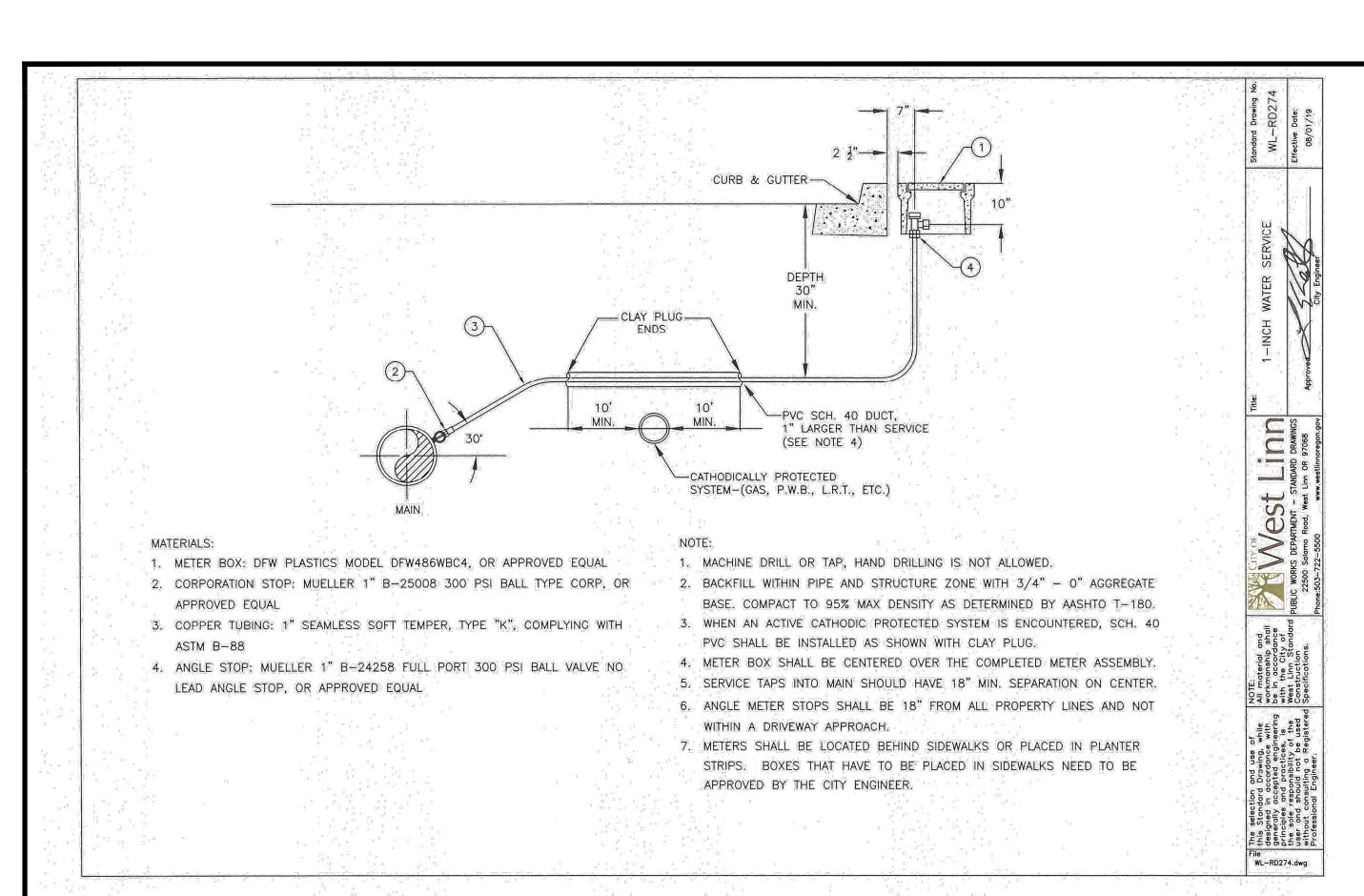
EXPIRES: 06/30/25 SIGNATURE DATE:

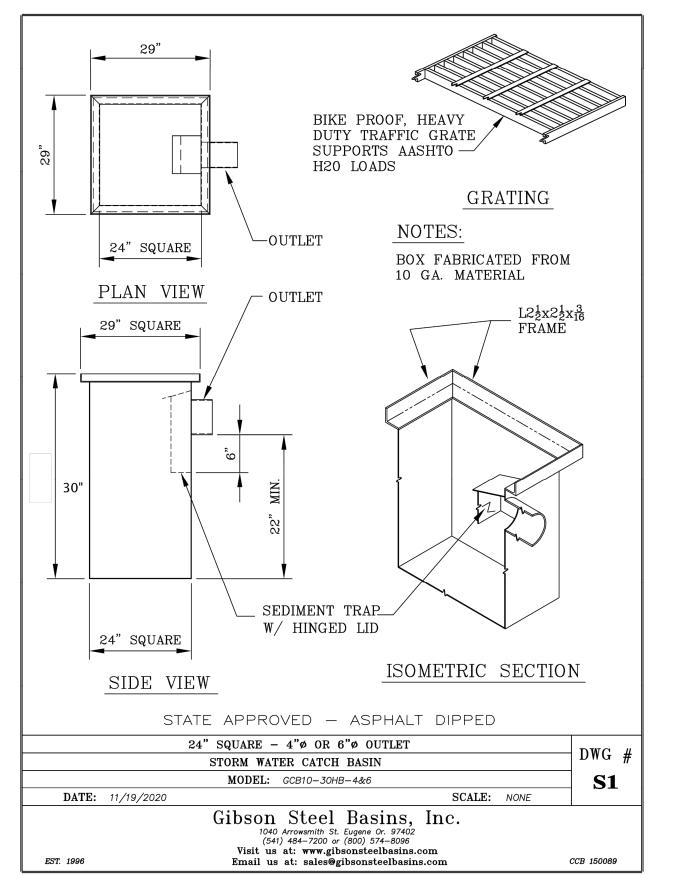
SCALE: AS SHOWN SEPTEMBER 2024 DATE: NO. REVISION PROJECT NO. E24-030				DRAWN:	JRF	DESIGNE	D: CMN	CHECKED: KAG	
DATE: NO. REVISION PROJECT NO. E24-030				SCALE:	AS S	HOWN	SEPTE	MBER 2024	
	DATE:	NO.	REVISION	PROJECT	Γ NO.	E24-030			

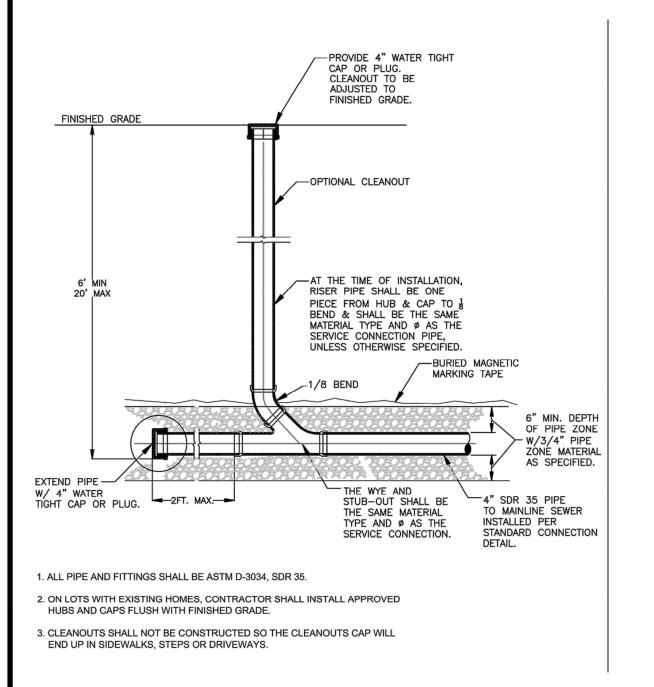


359 EAST HISTORIC COLUMBIA RIVER HIGHWAY TROUTDALE, OREGON 97060 (503) 668-3737

TPC PROPERTY LLC PO BOX 521 **DETAILS** DEVELOPMENT ENGINEERING PLANS West Linn, OR 97068







67149PE PRELIMINARY OREGON FROM A. GROVE EXPIRES: 06/30/25

STANDARD CLEANOUT

			DRAWN:	JRF	DESIGNE	D: CMN	CHECKED: KAG	
			SCALE:	AS S	HOWN	SEPTE	MBER 2024	
DATE:	NO.	REVISION	PROJECT	NO.	E24-030			



TPC PROPERTY LLC PO BOX 521 West Linn, OR 97068

DETAILS
DEVELOPMENT ENGINEERING PLANS

Preliminary Stormwater Report Cornwall 3 Lot Middle Housing City of West Linn, OR

September 2024

FDG Project No. E24-030



Prepared By:



FIRWOOD DESIGN GROUP, LLC

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Ш	FLOW CONTROL ANALYSIS	. 2
V	WATER QUALITY	. 3
VII	Down Stream Analysis	. 3
	Conclusion	

APPENDICES:

Appendix A: Stormwater and Grading Plan Appendix B: HydroCAD Output Appendix C: NRCS Soil Report Appendix D: Infiltration Test Report

EXISTING CONDITIONS

The project site is located at 4194 Cornwall Street, West Linn, OR (Parcel #21E36BA 065000), and is zoned R10. The current 0.49 acres lot features an existing residential structure and a detached garage. The site is relatively flat, with slopes less than 5%, gently sloping towards the southern side. The remainder of the lot is landscaped with mature trees and a retaining landscape wall along the southern property line. The site's soils are classified as "Saum Silt Loam" and fall within hydrological soil group C, with slopes ranging from 3 to 8 percent. See Figure 1 for details.



Figure 1: 4194 Cornwall Street, West Linn, OR 97068

II PROPOSED IMPROVEMENTS AND DESIGN PARAMETERS

The client proposes to partition the parcel into three parcels for single-family use. A new 14-foot wide, 130-foot long driveway is proposed to serve the first two lots, continuing as a 12-foot wide private driveway serving the third lot. The project will include the construction of utility services (water, sewer) to connect to existing mains on Cornwall Street. Stormwater improvements will include Lynch Style Catch Basin to capture driveway surface runoff for treatment, and drywells to fully infiltrate the Stormwater runoff from the catch basins and roof runoff directly without treatment; individual drywells are proposed for each private driveway and each private roof area runoff. See Stormwater and Grading Plan.

The design parameters applied are consistent with the City of West Linn Public Works Standards Section 2 and the current edition of the City of Portland Stormwater Management Manual.

Rapid Soil Solutions conducted a geotechnical infiltration test meeting the City standard and determined that the site soils are conducive to on-site infiltration.

Below is a summary of the findings. A copy of the full report is included in the appendices.

Location	Depth (ft)	Rate (in/hr.)
HA#1	3.5	6.375
HA#1	3.5	125

The design storms, as required by the City of West Linn and City of Portland design and construction standards, are as follows:

Recurrence Interval (years)	Total Precipitation Depth (In)
WQ	1.2
2	2.4
10	3.4
25	3.8

III FLOW CONTROL ANALYSIS

The Santa Barbara Urban Hydrography method (via HydroCAD) was used to create the basin hydrographs and estimate the peak flows for the 10-year design storms. For detailed data and calculations, please see the Hydrocad Output.

A curve number of 98 was assigned to all impervious surfaces, and a minimum time of concentration of 6 minutes was applied to all drainage basin areas.

The drywells were sized using the 10-year design storm per City standards.

The average infiltration rate derived from these tests is 3.812 inches per hour. For our stormwater design, we applied a safety factor of 2, resulting in an adjusted infiltration rate of 1.906 inches per hour. This rate was used in our stormwater calculations for the proposed drywells and planters. For the complete geotechnical report, please see Infiltration Test report attached.

The drywells were modeled assuming a 12-inch rock band surrounding the drywell with a 40% void ratio.

These basin areas and planter designs are discussed further in the next section. The following table summarizes the drywell design for the 10 year storm events.

Drywell	ID	Basin Area (sf)	Design Depth (ft)	Post Construction Storm Event	Peak Inflow (cfs)	Peak Elevation	Peak Storage (cf)
1	Shared Driveway	2,245	10'	10-year	0.04	12.17'	184
2	Lot 1 Private Driveway	930	5.5'	10-year	0.02	5.32'	64
3	Lot 1 Roof Area	2,175	9.5'	10-year	0.04	9.44'	178
4	Lot 2 Private Driveway	930	5.5'	10-year	0.02	5.32'	64
5	Lot 2 Roof Area	2,550	11.5'	10-year	0.05	11.26'	212
6	Lot 3 Private Driveway	960	6'	10-year	0.02	5.52'	67
7	Lot 3 Roof Area	3,016	14'	10-year	0.05	13.52'	255

The results of the analysis for full infiltration of the 10-year storm illustrate that a 36 inch and 48 -inch diameter with the specified design depth will be adequate, as the peak elevation does not exceed these depths during the maximum storm event.

V WATER QUALITY

Water quality treatment for all pollution generating surfaces is provided by Lynch style Catch basins as approved per Table 3-7 in the City of Portland Stormwater Management manual. Roof runoff is not required to be treated and will directly discharge into the proposed drywell systems.

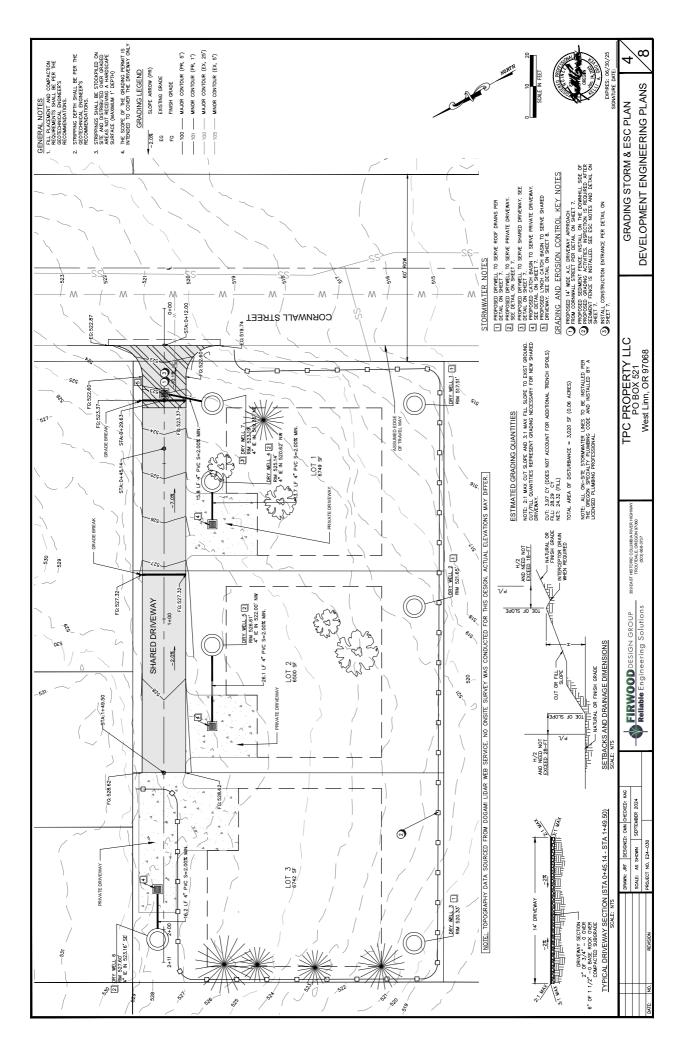
VII DOWN STREAM ANALYSIS

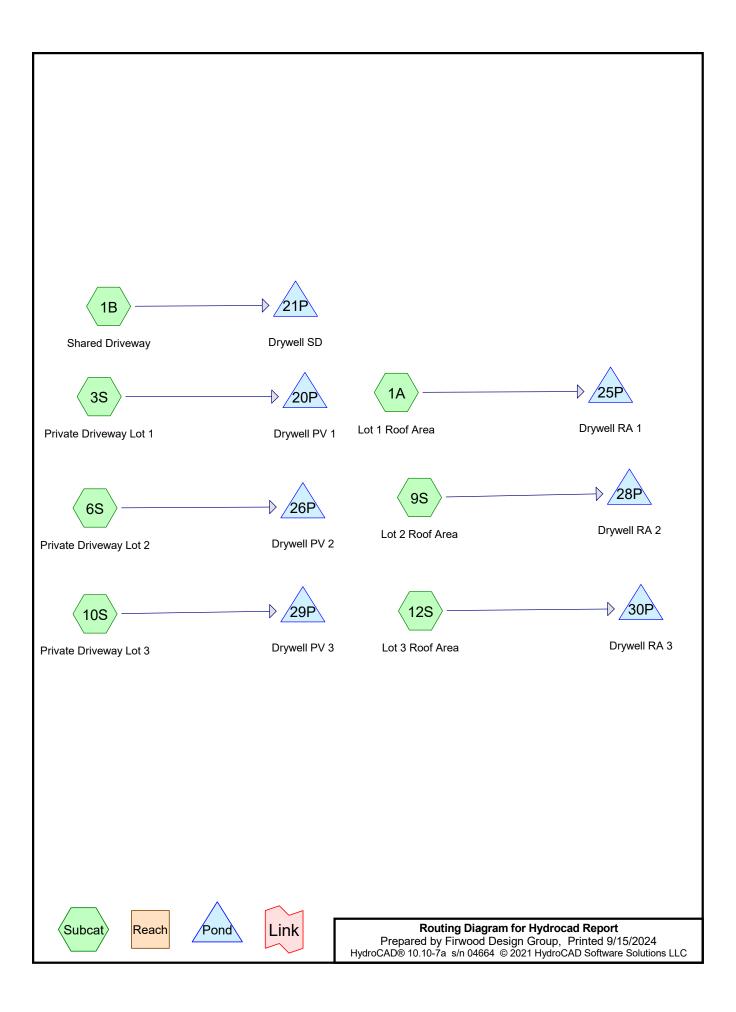
No downstream analysis is required as the site is 100% infiltrated on site per city of Portland Stormwater standards.

VIII CONCLUSION

The project site demonstrates favorable infiltration characteristics, enabling effective on-site stormwater management. The proposed stormwater management plan for the new improvements is summarized as follows: Stormwater from the proposed shared and private driveway areas will be treated by the proposed Lynch style catch basins.

Stormwater will be managed on-site through 100% infiltration for the 10-year design storm. The proposed stormwater management meets the City of West Linn Public Works standards.





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Rainfall Events Listing (selected events)

Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC
	Name				(hours)		(inches)	
1	10 Year	Type IA 24-hr		Default	24.00	1	3.40	2

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Area Listing (all nodes)

Area	CN	Description
 (sq-ft)		(subcatchment-numbers)
2,245	98	Asphalt (1B)
7,741	98	Building Roof Area (1A, 9S, 12S)
2,820	98	Concrete (3S, 6S, 10S)
12,806	98	TOTAL AREA

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Soil Listing (all nodes)

Area	Soil	Subcatchment
(sq-ft)	Group	Numbers
0	HSG A	
0	HSG B	
0	HSG C	
0	HSG D	
12,806	Other	1A, 1B, 3S, 6S, 9S, 10S, 12S
12,806		TOTAL AREA

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Ground Covers (all nodes)

	HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover	S N
Ī	0	0	0	0	2,245	2,245	Asphalt	
	0	0	0	0	7,741	7,741	Building Roof	
							Area	
	0	0	0	0	2,820	2,820	Concrete	
	0	0	0	0	12,806	12,806	TOTAL AREA	

Subcatch Numbers

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Time span=0.00-36.00 hrs, dt=0.01 hrs, 3601 points
Runoff by SBUH method, Split Pervious/Imperv.
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1A: Lot 1 Roof Area	Runoff Area=2,175 sf	100.00% Impervious	Runoff Depth=3.17"
----------------------------------	----------------------	--------------------	--------------------

Tc=6.0 min CN=0/98 Runoff=0.04 cfs 574 cf

Subcatchment 1B: Shared Driveway Runoff Area=2,245 sf 100.00% Impervious Runoff Depth=3.17"

Tc=6.0 min CN=0/98 Runoff=0.04 cfs 592 cf

Subcatchment 3S: Private Driveway Lot 1 Runoff Area=930 sf 100.00% Impervious Runoff Depth=3.17"

Tc=6.0 min CN=0/98 Runoff=0.02 cfs 245 cf

Subcatchment 6S: Private Driveway Lot 2 Runoff Area=930 sf 100.00% Impervious Runoff Depth=3.17"

Tc=6.0 min CN=0/98 Runoff=0.02 cfs 245 cf

Subcatchment 9S: Lot 2 Roof Area Runoff Area=2,550 sf 100.00% Impervious Runoff Depth=3.17"

Tc=6.0 min CN=0/98 Runoff=0.05 cfs 673 cf

Subcatchment 10S: Private Driveway Lot 3 Runoff Area=960 sf 100.00% Impervious Runoff Depth=3.17"

Tc=6.0 min CN=0/98 Runoff=0.02 cfs 253 cf

Subcatchment 12S: Lot 3 Roof Area Runoff Area=3,016 sf 100.00% Impervious Runoff Depth=3.17"

Tc=6.0 min CN=0/98 Runoff=0.05 cfs 796 cf

Pond 20P: Drywell PV 1 Peak Elev=5.32' Storage=64 cf Inflow=0.02 cfs 245 cf

Outflow=0.00 cfs 245 cf

Pond 21P: Drywell SD Peak Elev=9.78' Storage=184 cf Inflow=0.04 cfs 592 cf

Outflow=0.01 cfs 592 cf

Pond 25P: Drywell RA 1 Peak Elev=9.44' Storage=178 cf Inflow=0.04 cfs 574 cf

Outflow=0.01 cfs 574 cf

Pond 26P: Drywell PV 2 Peak Elev=5.32' Storage=64 cf Inflow=0.02 cfs 245 cf

Outflow=0.00 cfs 245 cf

Pond 28P: Drywell RA 2 Peak Elev=11.26' Storage=212 cf Inflow=0.05 cfs 673 cf

Outflow=0.01 cfs 673 cf

Pond 29P: Drywell PV 3 Peak Elev=5.52' Storage=67 cf Inflow=0.02 cfs 253 cf

Outflow=0.00 cfs 253 cf

Pond 30P: Drywell RA 3 Peak Elev=13.52' Storage=255 cf Inflow=0.05 cfs 796 cf

Outflow=0.01 cfs 796 cf

Total Runoff Area = 12,806 sf Runoff Volume = 3,379 cf Average Runoff Depth = 3.17" 0.00% Pervious = 0 sf 100.00% Impervious = 12,806 sf

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Summary for Subcatchment 1A: Lot 1 Roof Area

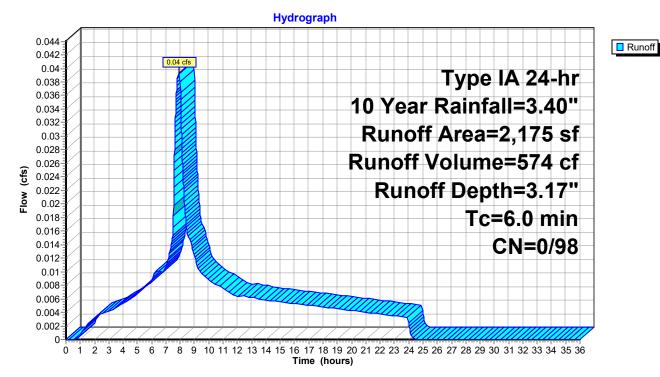
Runoff = 0.04 cfs @ 7.90 hrs, Volume= 574 cf, Depth= 3.17"

Routed to Pond 25P: Drywell RA 1

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type IA 24-hr 10 Year Rainfall=3.40"

_	Α	rea (sf)	CN [Description					
*		2,175	98 E	98 Building Roof Area					
		2,175	100.00% Impervious Area						
		Length	Slope	,		Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry, Minimum			

Subcatchment 1A: Lot 1 Roof Area



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Summary for Subcatchment 1B: Shared Driveway

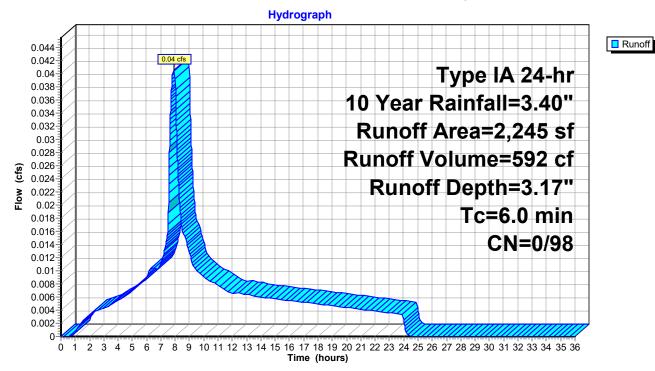
Runoff = 0.04 cfs @ 7.90 hrs, Volume= 592 cf, Depth= 3.17"

Routed to Pond 21P: Drywell SD

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type IA 24-hr 10 Year Rainfall=3.40"

	Α	rea (sf)	CN [Description		
*		2,245	98 <i>A</i>	\sphalt		
		2,245	100.00% Impervious Are			rea
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry, Minimum

Subcatchment 1B: Shared Driveway



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Summary for Subcatchment 3S: Private Driveway Lot 1

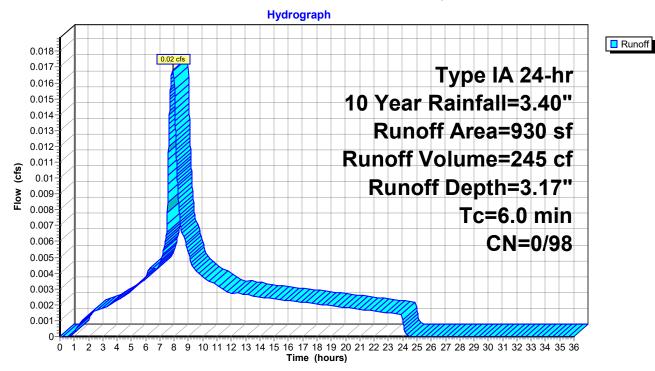
245 cf, Depth= 3.17" Runoff 0.02 cfs @ 7.90 hrs, Volume=

Routed to Pond 20P: Drywell PV 1

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type IA 24-hr 10 Year Rainfall=3.40"

_	Α	rea (sf)	CN [Description		
*		930	98 (Concrete		
		930	100.00% Impervious Area			
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry, Minimum

Subcatchment 3S: Private Driveway Lot 1



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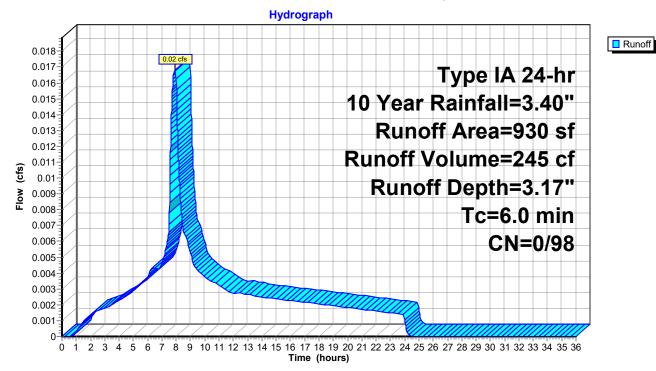
Summary for Subcatchment 6S: Private Driveway Lot 2

Runoff = 0.02 cfs @ 7.90 hrs, Volume= 245 cf, Depth= 3.17" Routed to Pond 26P : Drywell PV 2

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type IA 24-hr 10 Year Rainfall=3.40"

	Α	rea (sf)	CN [Description		
*		930	98 (Concrete		
		930	100.00% Impervious Ar			Area
	Tc	Length	Slope	,		Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry, Minimum

Subcatchment 6S: Private Driveway Lot 2



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Summary for Subcatchment 9S: Lot 2 Roof Area

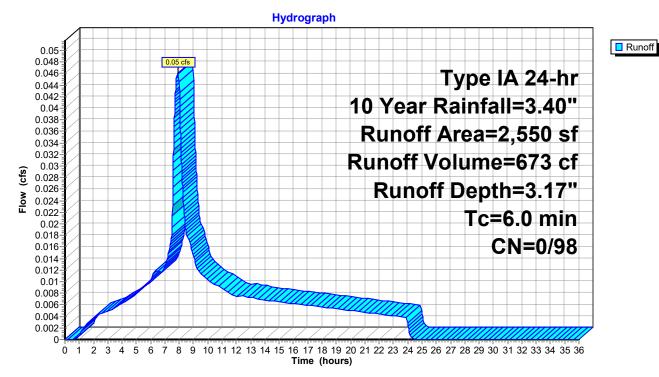
Runoff = 0.05 cfs @ 7.90 hrs, Volume= 673 cf, Depth= 3.17"

Routed to Pond 28P: Drywell RA 2

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type IA 24-hr 10 Year Rainfall=3.40"

	Α	rea (sf)	CN [Description					
*		2,550	98 E	98 Building Roof Area					
		2,550	100.00% Impervious Area						
	Tc	Length	Slope	,		Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry, Minimum			

Subcatchment 9S: Lot 2 Roof Area



Summary for Subcatchment 10S: Private Driveway Lot 3

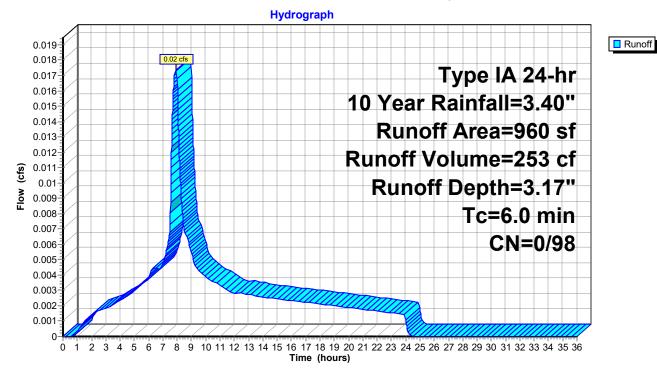
Runoff = 0.02 cfs @ 7.90 hrs, Volume= 253 cf, Depth= 3.17"

Routed to Pond 29P: Drywell PV 3

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type IA 24-hr 10 Year Rainfall=3.40"

	rea (sf)	CN E	Description		
*	960	98 C	Concrete		
	960	100.00% Impervious Area			Area
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	, ,		, ,	, ,	Direct Entry, Minimum

Subcatchment 10S: Private Driveway Lot 3



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Summary for Subcatchment 12S: Lot 3 Roof Area

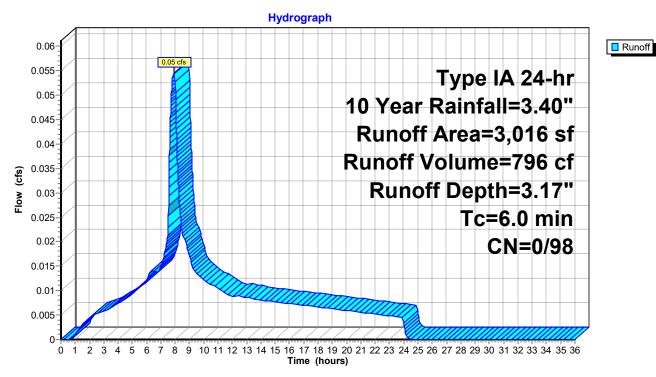
Runoff = 0.05 cfs @ 7.90 hrs, Volume= 796 cf, Depth= 3.17"

Routed to Pond 30P: Drywell RA 3

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type IA 24-hr 10 Year Rainfall=3.40"

	Α	rea (sf)	CN [Description				
*		3,016	98 E	98 Building Roof Area				
		3,016	,	100.00% Im	npervious A	rea		
		0	Slope	,		Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	6.0					Direct Entry, Minimum		

Subcatchment 12S: Lot 3 Roof Area



Inflow
■ Discarded

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Summary for Pond 20P: Drywell PV 1

Inflow Area = 930 sf,100.00% Impervious, Inflow Depth = 3.17" for 10 Year event

Inflow = 0.02 cfs @ 7.90 hrs, Volume= 245 cf

Outflow = 0.00 cfs @ 9.25 hrs, Volume= 245 cf, Atten= 73%, Lag= 81.0 min

Discarded = 0.00 cfs @ 9.25 hrs, Volume= 245 cf

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 5.32' @ 9.25 hrs Surf.Area= 20 sf Storage= 64 cf

Plug-Flow detention time= 191.7 min calculated for 245 cf (100% of inflow)

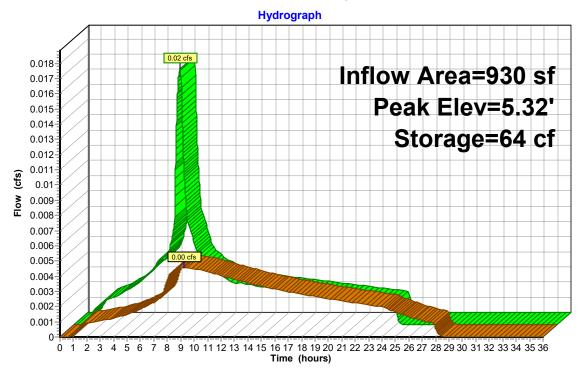
Center-of-Mass det. time= 191.8 min (856.3 - 664.6)

Volume	Invert	Avail.Storage	Storage Description
#1	0.00'	28 cf	5.00'D x 5.50'H Vertical Cone/Cylinder
			108 cf Overall - 39 cf Embedded = 69 cf x 40.0% Voids
#2	0.00'	39 cf	3.00'D x 5.50'H Vertical Cone/Cylinder Inside #1
		67 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.906 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.00 cfs @ 9.25 hrs HW=5.32' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.00 cfs)

Pond 20P: Drywell PV 1



InflowDiscarded

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Summary for Pond 21P: Drywell SD

Inflow Area = 2,245 sf,100.00% Impervious, Inflow Depth = 3.17" for 10 Year event

Inflow = 0.04 cfs @ 7.90 hrs, Volume= 592 cf

Outflow = 0.01 cfs @ 9.88 hrs, Volume= 592 cf, Atten= 77%, Lag= 119.3 min

Discarded = 0.01 cfs @ 9.88 hrs, Volume = 592 cf

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 9.78' @ 9.88 hrs Surf.Area= 28 sf Storage= 184 cf

Plug-Flow detention time= 286.8 min calculated for 592 cf (100% of inflow)

Center-of-Mass det. time= 286.9 min (951.5 - 664.6)

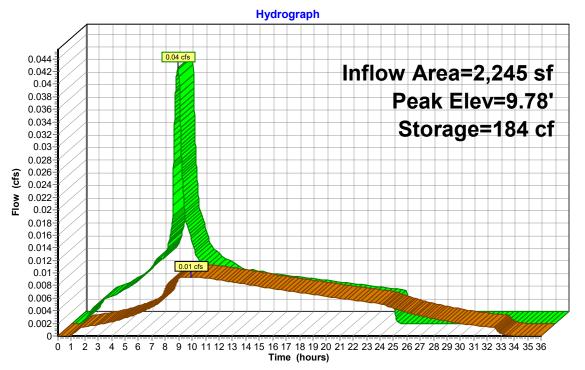
Volume	Invert	Avail.Storage	Storage Description
#1	0.00'	63 cf	6.00'D x 10.00'H Vertical Cone/Cylinder
			283 cf Overall - 126 cf Embedded = 157 cf x 40.0% Voids
#2	0.00'	126 cf	4.00'D x 10.00'H Vertical Cone/Cylinder Inside #1
		400 .5	Tatal Assilable Otenson

188 cf Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.906 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 9.88 hrs HW=9.78' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

Pond 21P: Drywell SD



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Summary for Pond 25P: Drywell RA 1

Inflow Area = 2,175 sf,100.00% Impervious, Inflow Depth = 3.17" for 10 Year event

Inflow = 0.04 cfs @ 7.90 hrs, Volume= 574 cf

Outflow = 0.01 cfs @ 9.88 hrs, Volume= 574 cf, Atten= 77%, Lag= 119.0 min

Discarded = 0.01 cfs @ 9.88 hrs, Volume= 574 cf

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 9.44' @ 9.88 hrs Surf.Area= 28 sf Storage= 178 cf

Plug-Flow detention time= 284.6 min calculated for 574 cf (100% of inflow)

Center-of-Mass det. time= 284.6 min (949.2 - 664.6)

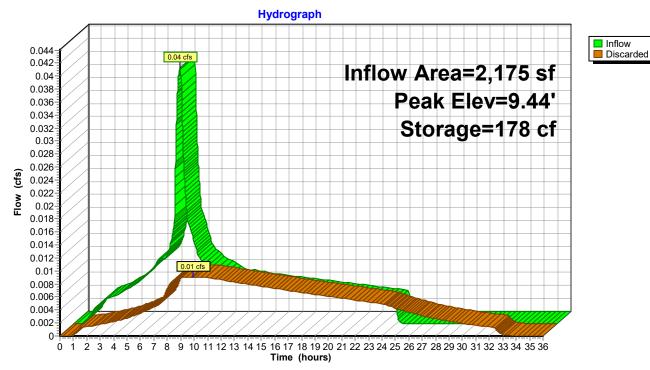
Volume	Invert	Avail.Storage	Storage Description
#1	0.00'	60 cf	6.00'D x 9.50'H Vertical Cone/Cylinder
			269 cf Overall - 119 cf Embedded = 149 cf x 40.0% Voids
#2	0.00'	119 cf	4.00'D x 9.50'H Vertical Cone/Cylinder Inside #1
		179 cf	Total Available Storage

Device Routing Invert Outlet Devices

#1 Discarded 0.00' 1.906 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 9.88 hrs HW=9.44' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

Pond 25P: Drywell RA 1



Inflow
■ Discarded

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Summary for Pond 26P: Drywell PV 2

Inflow Area = 930 sf,100.00% Impervious, Inflow Depth = 3.17" for 10 Year event

Inflow = 0.02 cfs @ 7.90 hrs, Volume= 245 cf

Outflow = 0.00 cfs @ 9.25 hrs, Volume= 245 cf, Atten= 73%, Lag= 81.0 min

Discarded = 0.00 cfs @ 9.25 hrs, Volume= 245 cf

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 5.32' @ 9.25 hrs Surf.Area= 20 sf Storage= 64 cf

Plug-Flow detention time= 191.7 min calculated for 245 cf (100% of inflow)

Center-of-Mass det. time= 191.8 min (856.3 - 664.6)

Volume	Invert	Avail.Storage	Storage Description
#1	0.00'	28 cf	5.00'D x 5.50'H Vertical Cone/Cylinder
			108 cf Overall - 39 cf Embedded = 69 cf x 40.0% Voids
#2	0.00'	39 cf	3.00'D x 5.50'H Vertical Cone/Cylinder Inside #1
		67 cf	Total Available Storage

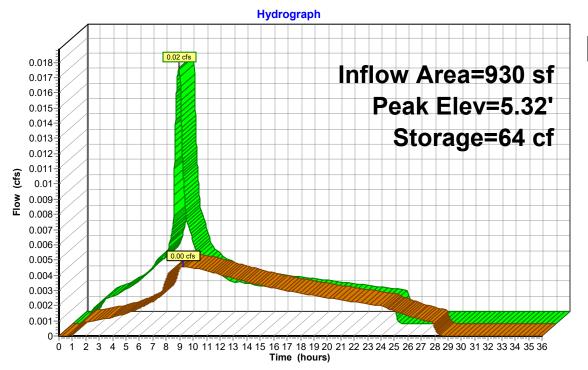
e Routing Invert Outlet Devices

Device Routing Invert Outlet Devices

#1 Discarded 0.00' 1.906 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.00 cfs @ 9.25 hrs HW=5.32' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.00 cfs)

Pond 26P: Drywell PV 2



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Summary for Pond 28P: Drywell RA 2

Inflow Area = 2,550 sf,100.00% Impervious, Inflow Depth = 3.17" for 10 Year event

Inflow = 0.05 cfs @ 7.90 hrs, Volume= 673 cf

Outflow = 0.01 cfs @ 9.90 hrs, Volume= 673 cf, Atten= 77%, Lag= 120.4 min

Discarded = 0.01 cfs @ 9.90 hrs, Volume= 673 cf

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 11.26' @ 9.90 hrs Surf.Area= 28 sf Storage= 212 cf

Plug-Flow detention time= 295.7 min calculated for 673 cf (100% of inflow)

Center-of-Mass det. time= 295.7 min (960.3 - 664.6)

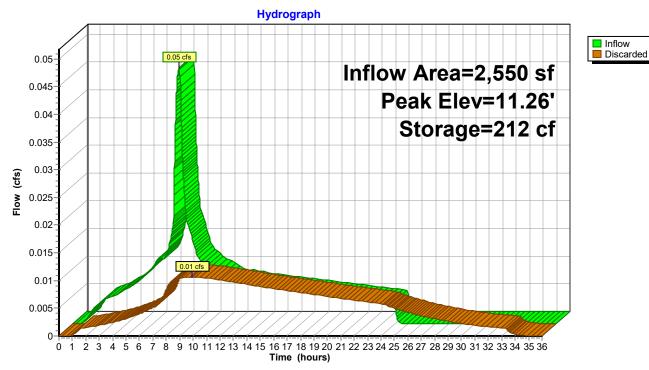
\	/olume	Invert	Avail.Storage	Storage Description
_	#1	0.00'	72 cf	6.00'D x 11.50'H Vertical Cone/Cylinder
				325 cf Overall - 145 cf Embedded = 181 cf x 40.0% Voids
_	#2	0.00'	145 cf	4.00'D x 11.50'H Vertical Cone/Cylinder Inside #1

217 cf Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.906 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 9.90 hrs HW=11.26' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

Pond 28P: Drywell RA 2



Hydrocad Report

Prepared by Firwood Design Group

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Inflow
■ Discarded

Summary for Pond 29P: Drywell PV 3

Inflow Area = 960 sf,100.00% Impervious, Inflow Depth = 3.17" for 10 Year event

Inflow = 0.02 cfs @ 7.90 hrs, Volume= 253 cf

Outflow = 0.00 cfs @ 9.25 hrs, Volume= 253 cf, Atten= 73%, Lag= 81.2 min

Discarded = 0.00 cfs @ 9.25 hrs, Volume= 253 cf

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 5.52' @ 9.25 hrs Surf.Area= 20 sf Storage= 67 cf

Plug-Flow detention time= 194.3 min calculated for 253 cf (100% of inflow)

Center-of-Mass det. time= 194.3 min (858.9 - 664.6)

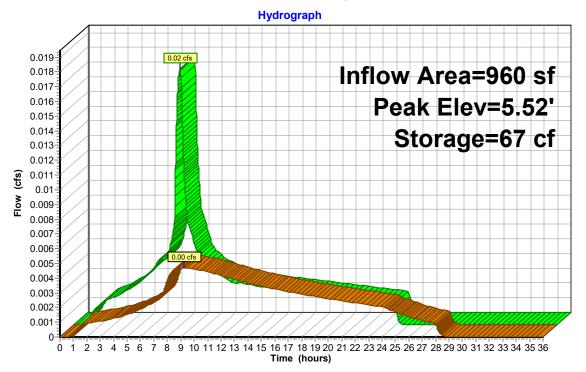
Volume	Invert	Avail.Storage	Storage Description
#1	0.00'	30 cf	5.00'D x 6.00'H Vertical Cone/Cylinder
			118 cf Overall - 42 cf Embedded = 75 cf x 40.0% Voids
#2	0.00'	42 cf	3.00'D x 6.00'H Vertical Cone/Cylinder Inside #1

73 cf Total Available Storage

evice)	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.906 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.00 cfs @ 9.25 hrs HW=5.52' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.00 cfs)

Pond 29P: Drywell PV 3



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Page 20

Inflow
■ Discarded

Summary for Pond 30P: Drywell RA 3

Inflow Area = 3,016 sf,100.00% Impervious, Inflow Depth = 3.17" for 10 Year event

Inflow = 0.05 cfs @ 7.90 hrs, Volume= 796 cf

Outflow = 0.01 cfs @ 9.92 hrs, Volume= 796 cf, Atten= 77%, Lag= 121.7 min

Discarded = 0.01 cfs @ 9.92 hrs, Volume= 796 cf

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 13.52' @ 9.92 hrs Surf.Area= 28 sf Storage= 255 cf

Plug-Flow detention time= 306.1 min calculated for 796 cf (100% of inflow)

Center-of-Mass det. time= 306.0 min (970.6 - 664.6)

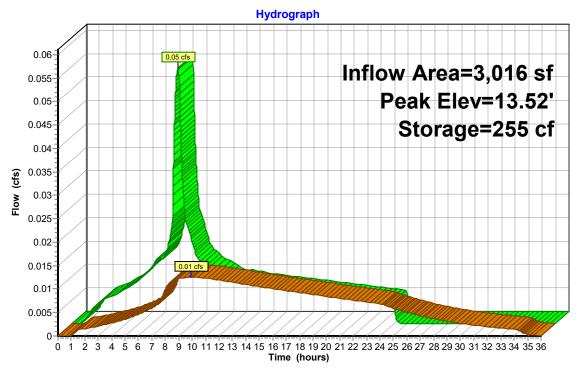
Volume	Invert	Avail.Storage	Storage Description
#1	0.00'	88 cf	6.00'D x 14.00'H Vertical Cone/Cylinder
			396 cf Overall - 176 cf Embedded = 220 cf x 40.0% Voids
#2	0.00'	176 cf	4.00'D x 14.00'H Vertical Cone/Cylinder Inside #1

264 cf Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.906 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 9.92 hrs HW=13.52' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

Pond 30P: Drywell RA 3





MAP LEGEND

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Water Features

Transportation

Background

Spoil Area

Stony Spot

Wet Spot

Other

Rails

US Routes

Major Roads

Local Roads

Very Stony Spot

Special Line Features

Streams and Canals

Interstate Highways

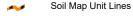
Aerial Photography

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Map Unit Polygons



Soil Map Unit Points

Special Point Features

Blowout

Borrow Pit

Clay Spot

Closed Depression

Gravel Pit

Gravelly Spot

Landfill

Lava Flow

Marsh or swamp

Mine or Quarry

Miscellaneous Water

Perennial Water

Rock Outcrop

→ Saline Spot

Sandy Spot

Severely Eroded Spot

Sinkhole

Slide or Slip

Sodic Spot

The soil surveys that comprise your AOI were mapped at 1:20.000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale

MAP INFORMATION

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Clackamas County Area, Oregon Survey Area Data: Version 20, Sep 7, 2023

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Sep 26, 2022—Oct 11, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.



MAP LEGEND MAP INFORMATION The soil surveys that comprise your AOI were mapped at Area of Interest (AOI) С 1:20.000. Area of Interest (AOI) C/D Soils Warning: Soil Map may not be valid at this scale. D Soil Rating Polygons Enlargement of maps beyond the scale of mapping can cause Not rated or not available Α misunderstanding of the detail of mapping and accuracy of soil **Water Features** line placement. The maps do not show the small areas of A/D Streams and Canals contrasting soils that could have been shown at a more detailed Transportation B/D Rails ---Please rely on the bar scale on each map sheet for map measurements. Interstate Highways C/D Source of Map: Natural Resources Conservation Service **US Routes** Web Soil Survey URL: D Major Roads Coordinate System: Web Mercator (EPSG:3857) Not rated or not available -Local Roads Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts Soil Rating Lines Background distance and area. A projection that preserves area, such as the Aerial Photography Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. B/D Soil Survey Area: Clackamas County Area, Oregon Survey Area Data: Version 20, Sep 7, 2023 Soil map units are labeled (as space allows) for map scales 1:50.000 or larger. Not rated or not available Date(s) aerial images were photographed: Sep 26, 2022—Oct 11. 2022 **Soil Rating Points** The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background A/D imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. B/D

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
78B	Saum silt loam, 3 to 8 percent slopes	С	0.6	100.0%
Totals for Area of Interest			0.6	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

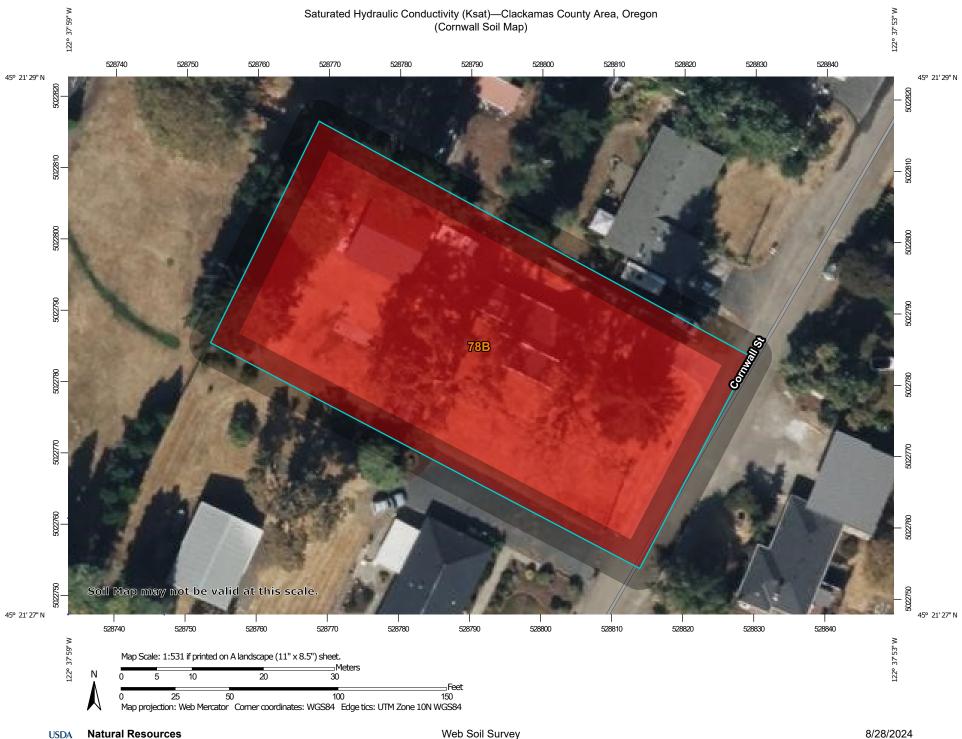
Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition
Component Percent Cutoff: None Specified

Tie-break Rule: Higher



MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Rating Polygons



= 9.0000



Not rated or not available

Soil Rating Lines



= 9.0000



Not rated or not available

Soil Rating Points



= 9.0000

Not rated or not available

Water Features



Streams and Canals

Transportation



Rails



Interstate Highways



US Routes



Major Roads



Local Roads

Background



Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20.000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Clackamas County Area, Oregon Survey Area Data: Version 20, Sep 7, 2023

Soil map units are labeled (as space allows) for map scales 1:50.000 or larger.

Date(s) aerial images were photographed: Sep 26, 2022—Oct 11, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Saturated Hydraulic Conductivity (Ksat)

Map unit symbol	Map unit name	Rating (micrometers per second)	Acres in AOI	Percent of AOI
78B	Saum silt loam, 3 to 8 percent slopes	9.0000	0.6	100.0%
Totals for Area of Interest			0.6	100.0%

Description

Saturated hydraulic conductivity (Ksat) refers to the ease with which pores in a saturated soil transmit water. The estimates are expressed in terms of micrometers per second. They are based on soil characteristics observed in the field, particularly structure, porosity, and texture. Saturated hydraulic conductivity is considered in the design of soil drainage systems and septic tank absorption fields.

For each soil layer, this attribute is actually recorded as three separate values in the database. A low value and a high value indicate the range of this attribute for the soil component. A "representative" value indicates the expected value of this attribute for the component. For this soil property, only the representative value is used.

The numeric Ksat values have been grouped according to standard Ksat class limits.

Rating Options

Units of Measure: micrometers per second
Aggregation Method: Dominant Component
Component Percent Cutoff: None Specified

Tie-break Rule: Fastest
Interpret Nulls as Zero: No

Layer Options (Horizon Aggregation Method): Depth Range (Weighted Average)

Top Depth: 36
Bottom Depth: 60

Units of Measure: Centimeters

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
78B	Saum silt loam, 3 to 8 percent slopes	0.6	100.0%
Totals for Area of Interest		0.6	100.0%

JJ Portlock 425-829-1566 jportlock@theportlockco.com

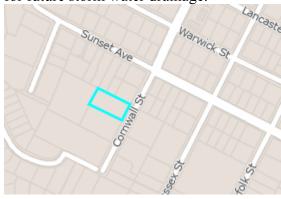
26 June 2024

Re: infiltration testing at 4194 Cornwall Street, West Linn OR

Dear Mr. Portlock,

Field Investigation:

Rapid Soil Solutions (RSS) has performed a two (2) infiltration tests. Figure 1 shows the project site location. The site has a gentle 5% slope to the rear where testing took place. Soils found on site match those in by DOGMI, RSS found fine- grained flood deposits. Testing was performed for future storm water drainage.



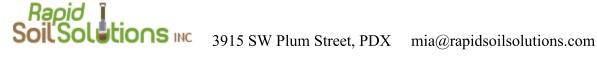
Infiltration Testing & Conclusion:

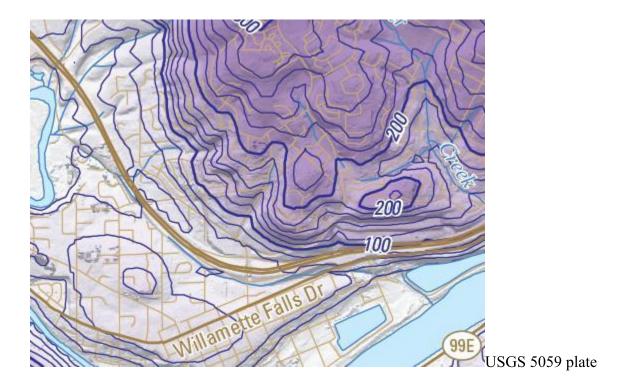
A summary of the test results is shown below. The attached sheets along with the soils and locations of the tests.

Location	Depth (ft)	Rate (in/hr.)
HA#1	3.5	6.375
HA#1	3.5	125

Depth to Ground Water

RSS was unable to excavate an addition hole to look for water at a depth of 10ft due to rock layer. No water loving vegetation was noted at the site. USGS has water mapped greater than 80ft below the surface.





The analysis, conclusions and recommendations contained in this report are based on site conditions as they existed at the time of explorations. Any questions regarding this report please contact me at the below number or email.

Sincerely,



Mia Mahedy, PE GE.

Rapid Soil Solutions Infiltration Test Results 23 X 1 * 26 🖃 0 4198 HA 1 HA₂ 4130 4195 4194 CORNWALL ST WEST LINN **Preliminary Information Performed By:** 4194 Cornwall St, West Location: (Supervised by Mia Mahedy, Rick Sands Linn, OR. PE, GE) Date & Time: **Instrument Used:** 6-24-24, 9:45 am 3 inch hand auger Weather: Depth: 3.5 ft 68, sunny HA#1 0-2 Dry, coarse, tan, silty clay, medium stiffness 2-3.5 Dry to damp, tan, coarse, silty clay, medium stiff to stiff Time Level Refilled To (inches) **Measurement (inches)** Rate (inches/hour) 10:25 13 5/8 9.5 10:45 11:05 7 11:25 5.25 13 3/8 11:45 10.25 7.75 12:05 12:25 6.25 13.5 12:45 10.75 1:05 9 1:25 7 1/8



Site Infiltration Rate (inches/hour)

6.375 in/hr.

Rapid Soil Solutions Infiltration Test Results 23 X 1 * 26 🖃 0 4198 HA 1 HA₂ 4130 4195 4194 CORNWALL ST WEST LINN **Preliminary Information Performed By:** 4194 Cornwall St, West Location: (Supervised by Mia Mahedy, Rick Sands Linn, OR. PE, GE) Date & Time: **Instrument Used:** 6-24-24, 9:45 am 3-inch hand auger Weather: 3.5 ft 68, sunny Depth: HA # 2 0-2 Dry, coarse, tan, silty clay, medium stiffness 2-3.5 Dry to damp, tan, coarse, silty clay, medium stiff to stiff Time Level Refilled To (inches) **Measurement (inches)** Rate (inches/hour) 10:25 12 7/8 11.5 10:45 11:05 10.75 11:25 10 3/8 13.25 11:45 12.75 12:05 12 1/8 12:25 11.75 13 1/8 12:45 12.5 1:05 12.25 1:25 11 7/8



Site Infiltration Rate (inches/hour)

1.25in/hr.